

United States Department of Agriculture
Natural Resources Conservation Service

OMB No. 0578-0030
NRCS-PDM-20

DAMAGE SURVEY REPORT (DSR)
Emergency Watershed Protection Program – Recovery

Section 1A

Date of Report: 5/2/2006

DSR Number: 051-05-022K Project Number: _____

NRCS Entry Only

Eligible: YES X NO _____
Approved: YES X NO _____
Funding Priority Number (from Section 4) 2e
Limited Resource Area: YES _____ NO X

Section 1B Sponsor Information

Sponsor Name: Jefferson Parish

Address: P.O. Box 10242

City/State/Zip: Jefferson, Louisiana 70554

Telephone Number: (504) 364-2700 Fax: _____

Section 1C Site Location Information

County: Jefferson State: Louisiana Congressional District: 02

Latitude: See Attached Map Longitude: See Attached Map Section: 42 Township: 12S Range: 10E

UTM Coordinates: 15

Drainage Name: Soniat Canal Reach: 1426 Feet, Sites 1, 2, 3a, 3b, 4, 5, 6, 7, & 8

Damage Description: Slope failure

Section 1D Site Evaluation

All answers in this Section must be YES in order to be eligible for EWP assistance.

Site Eligibility	YES	NO	Remarks
Damage was a result of a natural disaster?*	X		Hurricane Katrina Damage
Recovery measures would be for runoff retardation or soil erosion prevention?*	X		
Threat to life and/or property?*	X		
Event caused a sudden impairment in the watershed?*	X		
Imminent threat was created by this event?**	X		
For structural repairs, not repaired twice within ten years?***	X		
Site Defensibility			
Economic, environmental, and social documentation adequate to warrant action? (Go to pages 3, 4, 5 and 6 ***)	X		
Proposed action technically viable? (Go to Page 9 ***)	X		

Have all the appropriate steps been taken to ensure that all segments of the affected population have been informed of the EWP program and its possible effects? YES X NO _____

Comments: _____

* Statutory

** Regulation

*** DSR Pages 3 through 6 and 9 are required to support the decisions recorded on this summary page. If additional space is needed on this or any other page in this form, add appropriate pages. 1 of 14

DSR NO: 051-05-022K
Section 1E Proposed Action

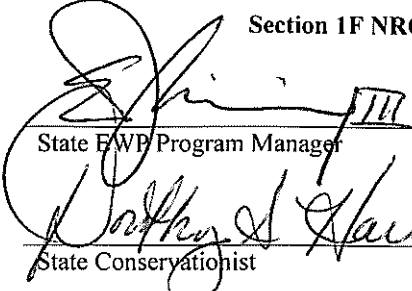
Describe the preferred alternative from Findings: Section 5 A:

The proposed action is the preferred alternative. The proposed action is the least costly alternative that achieves the restoration objectives, maximizes environmental benefits, and is socially acceptable.

Total installation cost identified in this DSR: Section 3: \$ _____

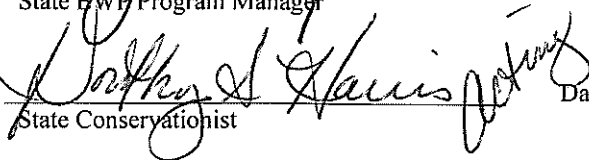
Section 1F NRCS State Office Review and Approval

Reviewed By:


State EWP Program Manager

Date Reviewed: 8/17/07

Approved By:


State Conservationist

Date Approved: 8/20/07

PRIVACY ACT AND PUBLIC BURDEN STATEMENT

NOTE: The following statement is made in accordance with the Privacy Act of 1974, (5 U.S.C. 552a) and the Paperwork Reduction Act of 1995, as amended. The authority for requesting the following information is 7 CFR 624 (EWP) and Section 216 of the Flood Control Act of 1950, Public Law 81-516, 33 U.S.C. 701b-1; and Section 403 of the Agricultural Credit Act of 1978, Public Law 95-334, as amended by Section 382, of the Federal Agriculture Improvement and Reform Act of 1996, Public Law 104-127, 16 U.S.C. 2203. EWP, through local sponsors, provides emergency measures for runoff retardation and erosion control to areas where a sudden impairment of a watershed threatens life or property. The Secretary of Agriculture has delegated the administration of EWP to the Chief or NRCS on state, tribal and private lands.

Signing this form indicates the sponsor concurs and agrees to provide the regional cost-share to implement the EWP recovery measure(s) determined eligible by NRCS under the terms and conditions of the program authority. Failure to provide a signature will result in the applicant being unable to apply for or receive a grant the applicable program authorities. Once signed by the sponsor, this information may not be provided to other agencies. IRS, Department of Justice, or other State or Federal Law Enforcement agencies, and in response to a court or administrative tribunal.

The provisions of criminal and civil fraud statutes, including 18 U.S.C. 286, 287, 371, 641, 651, 1001; 15 U.S.C. 714m; and 31 U.S.C. 3729 may also be applicable to the information provided. According to the Paperwork Reduction Act of 1995, an agency may not conduct or sponsor, and a person is not required to respond to a collection of information unless it displays a valid OMB control number. The valid OMB control number for this information collection is 0578-0030. The time required to complete this information collection is estimated to average 117/1.96 minutes/hours per response, including the time for reviewing instructions, searching existing data sources, field reviews, gathering, designing, and maintaining the data needed, and completing and reviewing the collection information.

USDA NONDISCRIMINATION STATEMENT

"The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.)

Persons with disabilities who require alternative means for communication of program information (Braille, large print, audiotope, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination write USDA, Director of Civil Rights, 1400 Independence Avenue, SW, Washington, DC 20250-9410 or call (800)795-3272 (voice) or (202)720-6382 (TDD). USDA is an equal opportunity provider and employer.

Civil Rights Statement of Assurance

The program or activities conducted under this agreement will be in compliance with the nondiscrimination provisions contained in the Titles VI and VII of the Civil Rights Act of 1964, as amended; the Civil Rights Restoration Act of 1987 (Public Law 100-259); and other nondiscrimination statutes: namely, Section 504 of the Rehabilitation Act of 1973, Title IX of the Amendments of 1972, the Age Discrimination Act of 1975, and the Americans with Disabilities Act of 1990. They will also be in accordance with regulations of the Secretary of Agriculture (7 CFR 15, 15a, and 15b), which provide that no person in the United States shall on the grounds of race, color, national origin, gender, religion, age or disability, be excluded from participation in, be denied the benefits of, or otherwise subjected to discrimination under any program or activity receiving Federal financial assistance from the U.S. Department of Agriculture or any agency thereof.

Section 2 Environmental Evaluation

2A Resource Concerns	2B Existing Condition	2C Alternatives and Effects		
		Proposed Action	No Action	Alternative
		Steel sheetpile toewall	No Repair	Concrete slopes
2D Effects of Alternatives				
Soil				
Soil Mass Movement	50 Tons/Acre	2 Tons/Acre	50 Tons/Acre	0 Tons/Acre
Water				
Downstream water rights	Will not have to drain channel to conduct repairs	No Effect	No Effect	No Effect
Turbidity	Increased turbidity decreasing water transparency	Long term improvement, short term increases in turbidity	Increased turbidity decreasing water transparency	Long term improvement, short term increases in turbidity
Air				
PM-10	Attainment	Attainment	Attainment	Attainment
Plant				
Aquatic Plant Health/Vigor	Poor due to decreased water transparency	Improved due to improved water transparency	Poor due to decreased water transparency	Improved due to improved water transparency
Emergent Health/Vigor	Poor due to steep slope and reduced emergent substrate	No emergent habitat with sheetpile toewall.	Poor due to steep slope and reduced emergent substrate	No emergent habitat with concrete slopes.
Riparian Vegetation	Not suitable to plants because of steep slope	No riparian habitat with sheetpile toewall.	Not suitable to plants because of steep slope	No riparian vegetation
Animal				
Fish habitat	Decreased due to increased turbidity and increased sedimentation	Improved water quality, decreased sedimentation	Decreased due to increased turbidity and increased sedimentation	Decreased due to elevated water temperatures and lack of riparian habitat.
Wading Bird Habitat	Decreased due to steep slope	Little change in shallow water habitat.	Decreased due to steep slope	Reduced wading bird habitat due to reduced riparian vegetation
Domestic and Migratory Waterfowl	Poor habitat due to steep slope	Narrow strip of riparian vegetation established above structure.	Poor habitat due to steep slope	Reduced waterfowl habitat due to reduced riparian vegetation
Other				
Maintenance	Unsafe to unstable slope	Improved	Unsafe to unstable slope	Improved
Aesthetics	Unightly exposed slope	Improved	Unightly exposed slope	Slight improvement to slight impairment

Section 2E Special Environmental Concerns

Resource Consideration	Existing Condition	Alternatives and Effects		
		Proposed Action	No Action	Alternative
Clean Water Act Waters of the U.S.	USACE Jurisdiction.	Improved water quality. Would require a CWA permit from USACE.	Decreased water quality. No permit required.	Improved water quality. Would require a permit CWA from USACE.
Coastal Zone Management Areas	The area is within the Coastal Zone Management Area (CZMA). Drainage impaired.	Consistency authorization needed.	No authorization needed.	Consistency authorization needed.
Coral Reefs	N/A (FOTG II)	N/A	N/A	N/A
Cultural Resources	(FOTG II) None observed onsite. Cross reference with state archeologist.	(FOTG II) None observed onsite. Cross reference with state archeologist..	(FOTG II) None observed onsite. Cross reference with state archeologist.	(FOTG II) None observed onsite. Cross reference with state archeologist.
Endangered and Threatened Species	(FOTG II) (Federal and State Lists) None observed, 10 known, 1 possible T&E species.	(FOTG II) (Federal and State Lists) None observed, 10 known, 1 possible T&E species	(FOTG II) (Federal and State Lists) None observed, 10 known, 1 possible T&E species	(FOTG II) (Federal and State Lists) None observed, 10 known, 1 possible T&E species
Environmental Justice	N/A (FOTG II)	N/A	N/A	N/A
Essential Fish Habitat	(FOTG II & NOAA) No EFH	(FOTG II & NOAA) No EFH	(FOTG II & NOAA) No EFH	(FOTG II & NOAA) No EFH
Fish and Wildlife Coordination	State level review with USFWS and LDWF. Consultation not required for slope repair.	State level review with USFWS and LDWF. Consultation not required for slope repair.	State level review with USFWS and LDWF. Consultation not required for slope repair.	State level review with USFWS and LDWF. Consultation not required for slope repair..
Floodplain Management	All project sites are located within the 100 year floodplain.	Maintenance of canals and levees prevents frequent flooding.	Maintenance of canals and levees prevents frequent flooding.	Maintenance of canals and levees prevents frequent flooding.
Invasive Species	Alligator weed and Johnsongrass is prevalent.	Increased spreading of Johnsongrass due to exposed slope.	Invasive plants remain and may spread without management.	Increased spreading of Johnsongrass due to exposed slope.
Migratory Birds	Mowed grass does not provide migratory bird habitat. Open water channel - no impact.	Mowed grass does not provide migratory bird habitat. Open water channel - no impact.	Mowed grass does not provide migratory bird habitat. Open water channel - no impact.	Mowed grass does not provide migratory bird habitat. Open water channel - no impact.
Natural Areas	None present (FOTG II)	None present (FOTG II)	None present (FOTG II)	None present (FOTG II)
Prime and Unique Farmlands	None present (FOTG II)	None present (FOTG II)	None present (FOTG II)	None present (FOTG II)
Riparian Areas	No riparian habitat is present due to steep bank.	Limited amount of riparian habitat could be established.	No riparian habitat is present due to steep bank.	Limited amount of riparian habitat could be established.
Scenic Beauty	None present (FOTG II)	None present (FOTG II)	None present (FOTG II)	None present (FOTG II)
Wetlands	Open water containing floating aquatics, algae, few emergent plants, litter. No impacts.	Open water containing floating aquatics, algae, few emergent plants, litter. No impacts.	Open water containing floating aquatics, algae, few emergent plants, litter. No impacts.	Open water containing floating aquatics, algae, few emergent plants, litter. No impacts.
Wild and Scenic Rivers	(FOTG II) (LDWF scenic streams list) Not Listed.	(FOTG II) (LDWF scenic streams list) Not Listed.	(FOTG II) (LDWF scenic streams list) Not Listed.	(FOTG II) (LDWF scenic streams list) Not Listed.

Completed By: Gene BarickmanDate: 5/11/2006

DSR NO: 051-05-022K
Section 2F Economic

This section must be completed by each alternative considered (attach additional sheets as necessary).

	Future Damages (\$)	Damage Factor (%)	Near Term Damage Reduction
Properties Protected (Private)			
Houses – 100 @ 200,000	20,000,000.00	20	4,000,000.00
Commercial Businesses – 10 @ 500,000	5,000,000.00	20	1,000,000.00
Apartment Units – 20 @ 30,000	600,000.00	20	120,000.00
Properties Protected (Public)			
Court House – 1 @ 3,000,000.00	3,000,000.00	20	600,000.00
Business Losses			
Other			
Total Near Term Damage Reduction \$			5,720,000.00
Net Benefit (Total Near Term Damage Reduction minus cost from section 3)			

Completed By: Alan Bolotte Date: 05/02/2006

Section 2G Social Consideration

This section must be completed by each alternative considered (attach additional sheets as necessary).

	YES	NO	Remarks
Has there been a loss of life as a result of the watershed impairment?		X	
Is there the potential for loss of life due to damages from the watershed impairment?	X		
Has access to a hospital or medical facility been impaired by watershed impairment?	X		
Has the community as a whole been adversely impacted by the watershed impairment (life and property ceases to operate in a normal capacity)	X		The potential for flooding in this area is high and if flooding were to occur the community as a whole would be affected.
Is there a lack or has there been a reduction of public safety due to watershed impairment?	X		

Completed By: Alan Bolotte Date: 05/02/2006

Section 2H Group Representation and Disability Information**This section is completed only for the preferred alternative selected.**

Group Representation	Number
American Indian/Alaska Native Female Hispanic	
American Indian/Alaska Native Female Non-Hispanic	
American Indian/Alaska Native Male Hispanic	
American Indian/Alaska Native Male Non-Hispanic	
Asian Female Hispanic	
Asian Female Non-Hispanic	6
Asian Male Hispanic	
Asian Male Non-Hispanic	5
Black or African American Female Hispanic	
Black or African American Female Non-Hispanic	48
Black or African American Male Hispanic	
Black or African American Male Non-Hispanic	
Hawaiian Native/Pacific Islander Female Hispanic	45
Hawaiian Native/Pacific Islander Female Non-Hispanic	
Hawaiian Native/Pacific Islander Male Hispanic	
Hawaiian Native/Pacific Islander Male Non-Hispanic	
White Female Hispanic	16
White Female Non-Hispanic	123
White Male Hispanic	14
White Male Non-Hispanic	114
Total Group	370

Census tract(s) _ U.S. Census Bureau, American FactFinder, Jefferson Parish

Completed By: ___ Alan Bolotte revised by J Burwell
12/07/06

Date: ___ 05/02/2006 _____

Section 2I. Required consultation or coordination between the lead agency and/or the RFO and another governmental unit including tribes:

Easements, permissions, or permits:

Access permission will be required.

CWA permit prior to construction in channel is needed.

Water Quality Certification is needed.

A consistency authorization is needed since project location is in the coastal zone.

Mitigation Description:

Construction will be done in consecutive days to minimize impacts to local wildlife.

Re-seed construction areas to NRCS Standard and Specification 342 to minimize run-off effects and provide vegetative riparian buffer.

Agencies, persons, and references consulted, or to be consulted:

U.S. Army Corps of Engineers

LDWF

Jefferson Parish

LDNR

Private Landowners

LDEQ

Section 3 Engineering Cost Estimate

Completed By: _____ Marty Comstock (Rev. C. LaFleur) _____ Date: ____5/13/06_____

This section must be completed by each alternative considered (attach additional sheets as necessary).

Proposed Recovery Measure (including mitigation)	Quantity	Units	Unit Cost (\$)	Amount (\$)
Site 1 – Steel Sheet Piling Slope Repair	260	LF		
Site 2 – Steel Sheet Piling Slope Repair	186	LF		
Site 3a – Steel Sheet Piling Slope Repair	136	LF		
Site 3b – Steel Sheet Piling Slope Repair	20	LF		
Site 4 – Steel Sheet Piling Slope Repair	80	LF		
Site 5 - Steel Sheet Piling Slope Repair	70	LF		
Site 6 - Steel Sheet Piling Slope Repair	480	LF		
Site 7 - Steel Sheet Piling Slope Repair	52	LF		
Site 8 - Steel Sheet Piling Slope Repair	142	LF		
Total Installation Cost (Enter in Section 1F)\$				

Unit Abbreviations:

AC Acre
 CY Cubic Yard
 EA Each
 HR Hour
 LF Linear Feet
 LS Lump Sum
 SF Square Feet
 SY Square Yard
 TN Ton
 Other (Specify)

Section 4 NRCS EWP Funding Priority

Complete the following section to compute the funding priority for the recovery measures in this application (see instructions on page 10).

Priority Ranking Criteria	Yes	No		Ranking Number Plus Modifier
1. Is this an exigency situation?		x		
2. Is this a site where there is serious, but not immediate threat to human life?	x			2E
3. Is this a site where buildings, utilities, or other important infrastructure components are threatened?		x		
4. Is this site a funding priority established by the NRCS Chief?		x		
The following are modifiers for the above criteria			Modifier	
a. Will the proposed action or alternatives protect or conserve federally-listed threatened and endangered species or critical habitat?				
b. Will the proposed action or alternatives protect or conserve cultural sites listed on the National Register of Historic Places?				
c. Will the proposed action or alternatives protect or conserve prime or important farmland?				
d. Will the proposed action or alternatives protect or conserve existing wetlands?				
e. Will the proposed action or alternatives maintain or improve current water quality conditions?			e	
f. Will the proposed action or alternatives protect or conserve unique habitat, including but not limited to, areas inhabited by State-listed species, fish and wildlife management area, or State identified sensitive habitats?				

Enter priority computation in Section 1A, NRCS Entry, Funding priority number.

Remarks:

Section 5A Findings

Finding: Indicate the preferred alternative from Section 2 (Enter to Section 1E):


The proposed action is debris removal. The proposed action is the preferred alternative. The proposed action is the least costly alternative that achieves the restoration objectives, maximizes environmental benefits, and is socially acceptable.

I have considered the effects of the action and the alternatives on the Environmental Economic, Social; the Special Environmental Concerns; and the extraordinary circumstances (40 CFR 1508.27). I find for the reasons stated below, that the preferred alternative:

☒ Has been sufficiently analyzed in the EWP PEIS (reference all that apply)
Chapter 5.2.2.2.2
Chapter _____
Chapter _____
Chapter _____
Chapter _____

☐ May require the preparation of an environmental assessment or environmental impact statement.
The action will be referred to the NRCS State Office on this date:

NRCS representative of the DSR team

Title:  DC Date: 7/12/06

Section 5B Comments:

Section 5C

Sponsor Concurrence: _____

Sponsor Representative

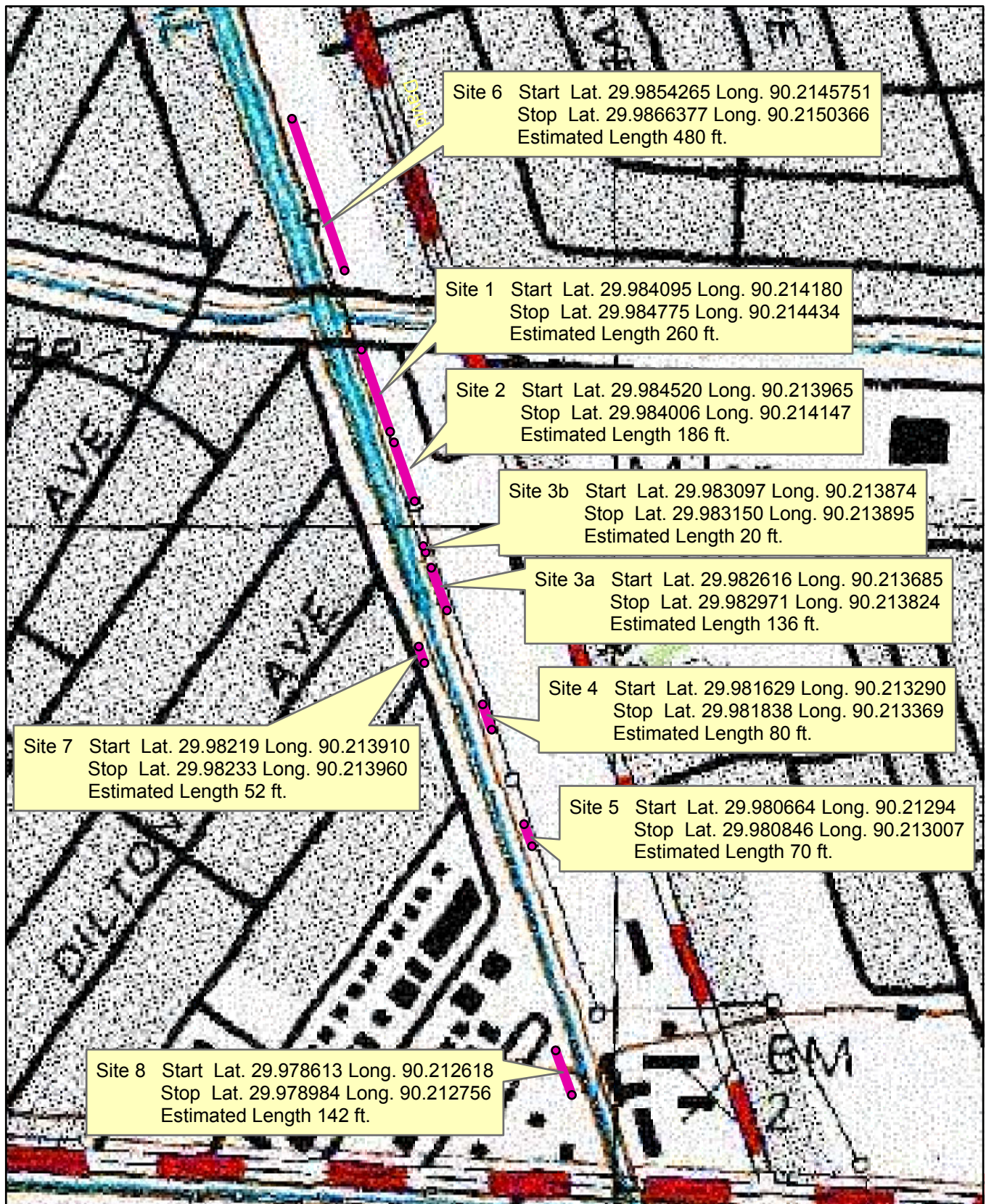
Title:  Date: 7/12/06

Section 6 Attachments:

- A. Location Map
- B. Site Plan or Sketches
- C. Other (explain)

Jefferson Parish

DSR # 051-05-022K - Soniat Canal Sites 1- 8



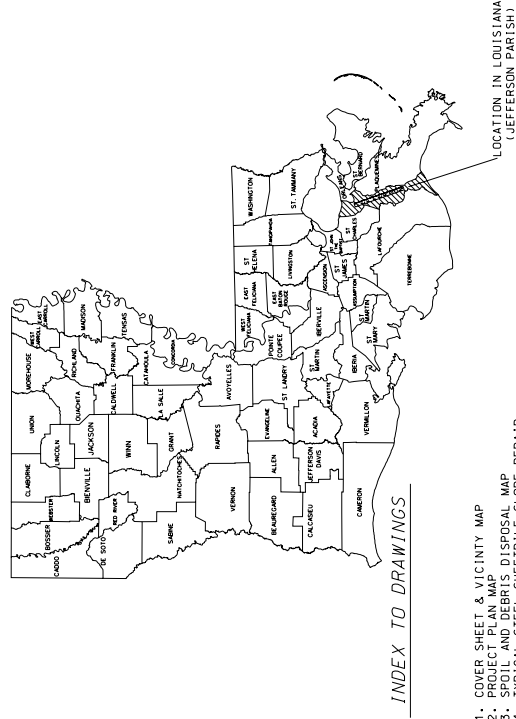
0 262.5 525 1,050
Feet



VICINITY MAP
DSR-051-05-022K



DSR-051-05-022K
SONIAT CANAL
EMERGENCY REPAIR
BUILT UNDER THE
EMERGENCY WATERSHED PROTECTION PROGRAM
BY JEFFERSON PARISH
WITH THE ASSISTANCE OF THE
NATURAL RESOURCES CONSERVATION SERVICE
OF THE
UNITED STATES DEPARTMENT OF AGRICULTURE
2007



INDEX TO DRAWINGS

- 1. COVER SHEET & VICINITY MAP
- 2. PROJECT PLAN MAP
- 3. SPOIL AND DEBRIS DISPOSAL MAP
- 4. TYPICAL STEEL SHEETPILE SLOPE REPAIR
- 5. DETAILS - PIPE SUPPORT
- 6. DETAILS - PIPE SUPPORT
- 7. SITE PLAN - SITE #1
- 8. SITE #1 - SECTIONS - STA. 0+36 & 0+68
- 9. SITE #1 - SECTIONS - STA. 1+50 & 2+17
- 10. SITE #1 - SECTION - STA. 2+42
- 11. SITE #1 - SECTIONS - STA. 0+00 & 0+50
- 12. SITE #1 - SECTIONS - STA. 0+61 & 1+01
- 13. SITE #2 - SECTIONS - STA. 1+23 & 1+67
- 14. SITE #2 - SECTIONS - STA. 1+23 & 1+67
- 15. SITE PLAN - SITES #3A & 3B
- 16. SITE #3A - SECTIONS - STA. 0+00 & 0+09
- 17. SITE #3A - SECTIONS - STA. 0+54 & 1+04
- 18. SITE #3A - SECTIONS - STA. 0+95 & 1+09
- 19. SITE #3A/B - SECTIONS - STA. 1+36 & 1+95
- 20. SITE PLAN - SITE #4
- 21. SITE #4 - SECTIONS - STA. 0+31 & 0+55
- 22. SITE #4 - SECTIONS - STA. 0+55 & 0+57
- 23. SITE #5 - SECTIONS - STA. 0+40 & 0+25
- 24. SITE #5 - SECTIONS - STA. 0+48 & 0+68
- 25. SITE PLAN - SITE #6
- 26. SITE #6 - SECTIONS - STA. 0+45 & 1+49
- 27. SITE #6 - SECTIONS - STA. 1+49 & 1+57
- 28. SITE #6 - SECTIONS - STA. 3+11 & 3+52
- 29. SITE #6 - SECTION - STA. 4+57
- 30. SITE PLAN - SITE #7
- 31. SITE #7 - SECTION - STA. 1+82
- 32. SITE PLAN - SITE #8
- 33. SITE #8 - SECTIONS - STA. 0+00 & 0+57
- 34. SITE #8 - SECTIONS - STA. 1+03 & 1+41



FILE NAME
LA051-05-022K-01
DRAWING NAME
LA051-05-022K-01

DESIGNED	J. MOORE
DRAWN	ABBEY GREWMILLION
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING III
DATE	3/07
	4/07
	4/07
	4/07

COVER SHEET & VICINITY MAP
EWP - HURRICANE KATRINA
DSR051-05-022K - SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

NOTICE:
96 HOURS BEFORE DIGGING
CALL 1-800-272-3020
TO LOCATE UTILITY LINES

TBM SONIAT CANAL
PAINTED 'X' ON SOUTHEAST CORNER
OF CONCRETE SLAB NEXT TO CHANNEL
N=541894.305
E=3634967.577
ELEV.=20.00(ASSUMED)

ACCESS TO SITE #6 SHALL BE FROM
CAMPHOR ST. OFF OF DAVID DR.



ESTIMATED QUANTITIES

SITE #	1	2	3A	3B	4	5	6	7	8
EXCAVATION (CY)	1848	1570	648	152	744	347	2653	267	1110
SIZE #1 ROCKFILL (CY)	964	731	383	57	270	142	1430	137	299
SIZE #57 ROCKFILL (CY)	765	848	419	62	515	236	1072	125	715
TOP SOIL (CY)	232	213	125	18	139	92	553	47	170
GEOTEXTILE (SY)	2398	1763	1184	174	758	277	4534	433	1449
ECRM (SY)	834	632	437	60	329	296	1537	152	553
TOEWALL-STEEL (LF)	260	186	136	20	80	70	480	52	142

PROJECT MAP - SONIAT CANAL
DSR-051-005-022K - JEFFERSON PARISH



SHEET 02 of 34

FILE NAME
DSR-051-05-022K-02
DRAWING NAME
DSR-051-05-022K-02



PROJECT PLAN MAP
EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

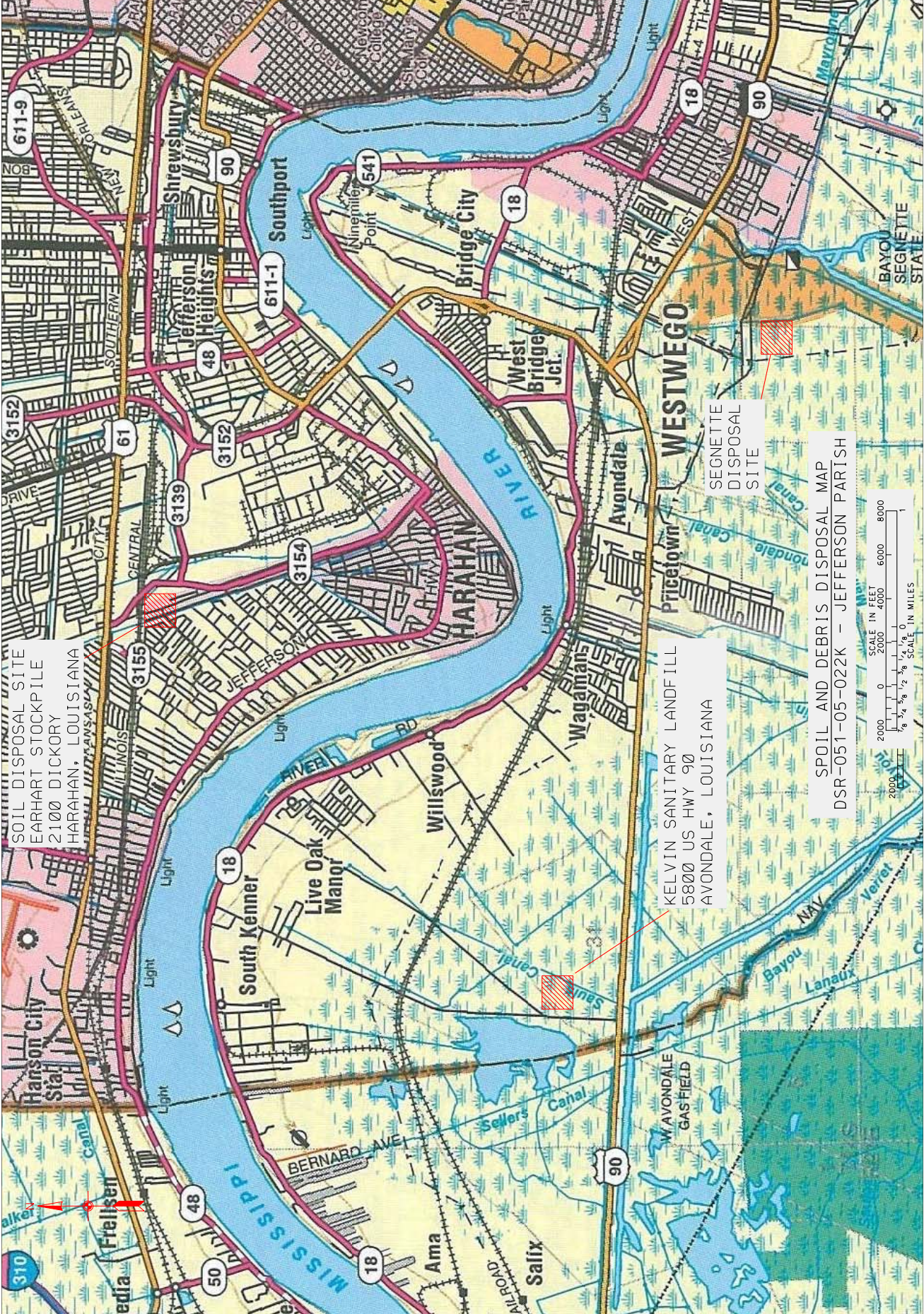
DESIGNED	JIMMY MOORE	DATE	03/07
DRAWN	ABBEY GREMILLION		04/07
CHECKED	CHERIE LAFLEUR		04/07
APPROVED	ED GIERING III		04/07

DATE	3/07
DESIGNED BY	JIMMY MOORE
DRAWN BY	ABBEY GREWMILLION
CHECKED BY	CHERIE LAFLEUR
APPROVED BY	ED GIERING, III
	4/07

SPoil AND DeBRIS DiSPoSal MAP
 EWP - HURRICANE KATRINA
 DSR051-05-022K-SOUIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA-051-05-022K-03
DRAWING NAME	LA-051-05-022K-03

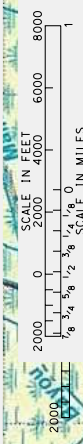


SOIL DISPOSAL SITE
 EARHART STOCKPILE
 2100 DICKORY
 HARAHAN, LOUISIANA

KELVIN SANITARY LANDFILL
 5800 US HWY 90
 AVONDALE, LOUISIANA

SEGNETTE
 DISPOSAL
 SITE

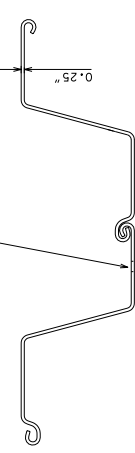
SPoil AND DeBRIS DiSPoSal MAP
 DSR-051-05-022K - JEFFERSON PARISH



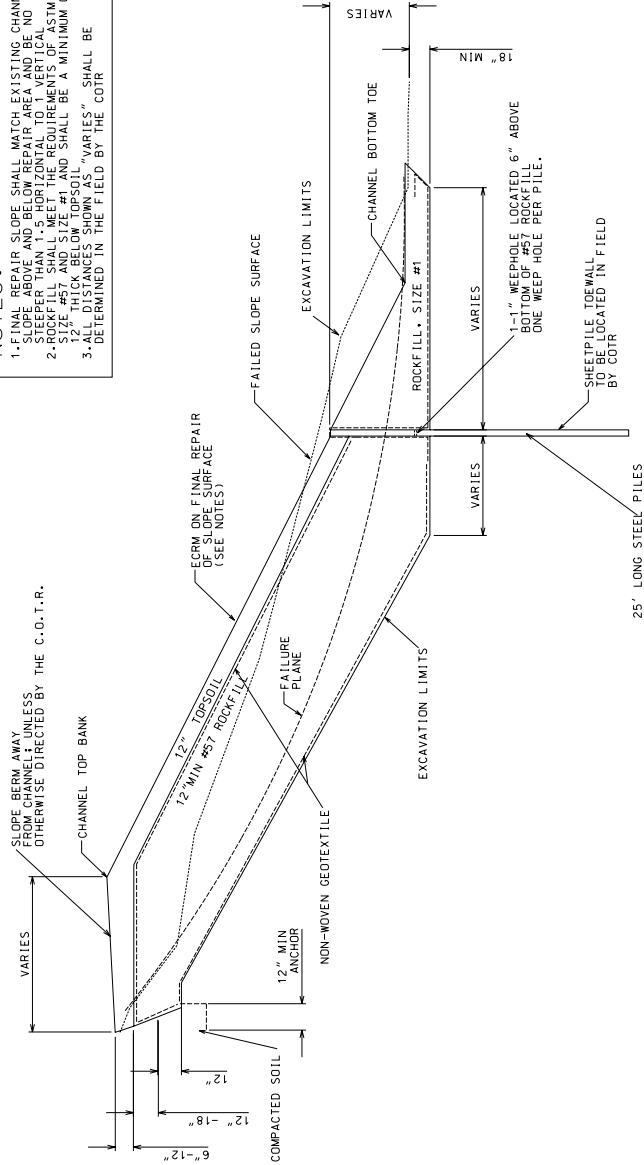
NOT TO SCALE

- NOTES:
1. STEEL SHEET PILING SHALL CONFORM TO ASTM A328 AND HAVE A MINIMUM SECTION MODULUS OF 13.00 CU. IN. PER LINEAR FOOT OF SHEET PILE WALL.
 2. MINIMUM WEB AND FLANGE THICKNESS SHALL BE 0.25 INCHES.
 3. STEEL SHEET PILING SHALL BE DRIVEN IN THE "NORMAL CONFIGURATION" AS SHOWN.

- NOTES:
1. FINAL REPAIR SLOPE SHALL MATCH EXISTING CHANNEL SLOPE ABOVE AND BELOW REPAIR AREA AND BE NO STEEPER THAN 1.5 HORIZONTAL TO 1 VERTICAL.
 2. SLOPE SHALL BE REINFORCED WITH 12" MIN. TOPSOIL C-33, 12" MIN. #57 ROCKFILL, AND 12" MIN. #41 AND SHALL BE A MINIMUM OF 12" THICK BELOW TOPSOIL.
 3. ALL DISTANCES SHOWN AS "VARIES" SHALL BE DETERMINED IN THE FIELD BY THE COIR.

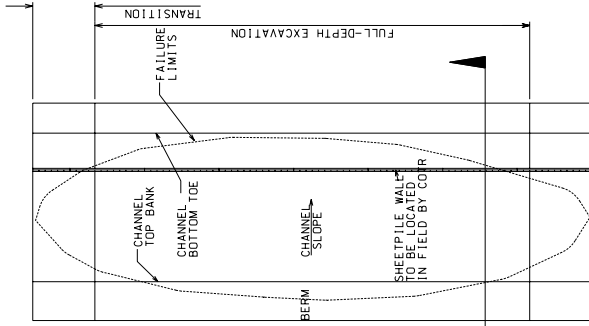


TYPICAL SHEETPILE DETAIL
(NORMAL CONFIGURATION)



NOTICE:
96 HOURS BEFORE DIGGING
CALL 1-800-272-3020
TO LOCATE UTILITY LINES

CHANNEL SLOPE REPAIR
SHEETPILE TOEWALL



SHEETPILE TOE WALL REPAIR LOCATIONS	
CHANNEL NAME	DSR # SITES
SONIAT CANAL	051-05-022K 1-8

EXISTING SHEETPILE TOE WALL REMOVAL LOCATIONS	
CHANNEL NAME	SITE LENGTH TO BE REMOVED
SONIAT CANAL	SITE #4 35 FEET
SONIAT CANAL	SITE #8 140 FEET

CHANNEL SLOPE REPAIR
PLAN



FILE NAME
LA051-05-022K-04
DRAWING NAME
LA051-05-022K-04

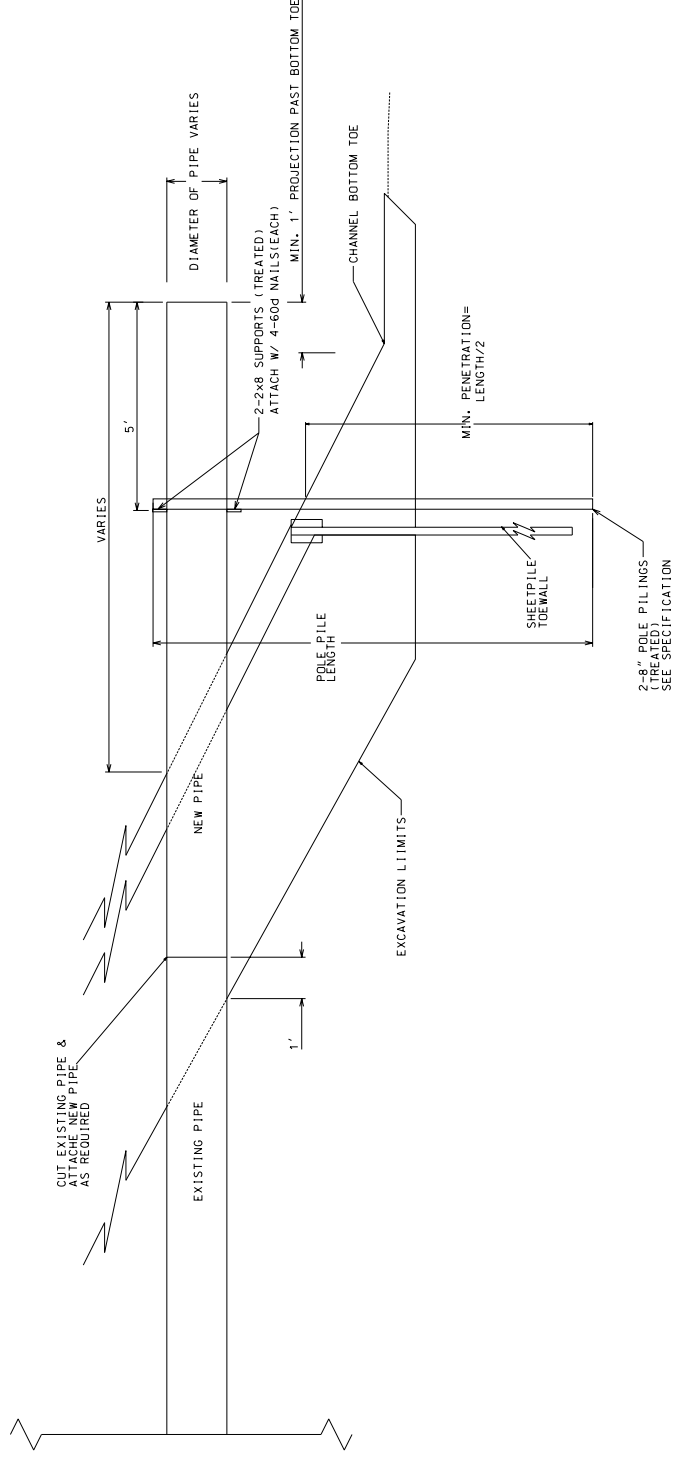
Sheet 04 of 34

TYPICAL STEEL SHEETPILE SLOPE REPAIR
EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

DESIGNED JIMMY MOORE
DRAWN ABBEY GREMILLION
CHECKED CHERIE LAFLEUR
APPROVED ED CIERING, III
DATE 03/07 04/07 04/07 04/07

CHANNEL NAME	DSR #	SITE #	STA.	DIA.	MATERIAL	REMOVE	ADD
SONIAT CANAL	DSR-051-05-022K	1	0+36	12"	CMP	21'	21'
SONIAT CANAL	DSR-051-05-022K	2	0+60	12"	CMP	20'	25'
SONIAT CANAL	DSR-051-05-022K	3A	0+74	18"	PVC	16'	21'
SONIAT CANAL	DSR-051-05-022K	3A	0+97	12"	PVC	12'	21'
SONIAT CANAL	DSR-051-05-022K	8	1+41	12"	PVC	27'	31'
SONIAT CANAL	DSR-051-05-022K	1	2+17	15"	CMP	15'	20'

NOTICE:
96 HOURS BEFORE DIGGING
CALL 1-800-272-3020
TO LOCATE UTILITY LINES



NOT TO SCALE

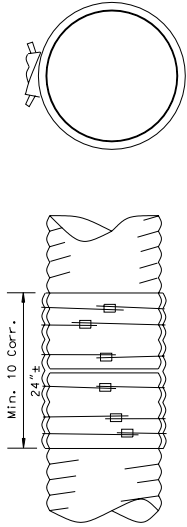
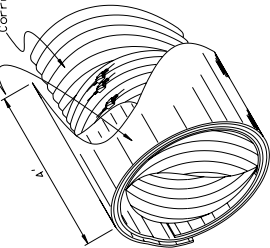
REVISIONS		
NO.	DATE	TITLE

NRCS
Natural Resources Conservation Service
United States Department of Agriculture

DATE	DESIGNED	DRAWN	CHECKED	APPROVED
03/07	J. MOORE	ABBEY GREMILLION	CHERIE LAFLEUR	ED GIERING III
04/07				
04/07				

DATE	03/07	04/07	04/07	04/07
------	-------	-------	-------	-------

Geotextile (2 Laps Required - Over Connecting Band)
Corrugated Metal Pipe

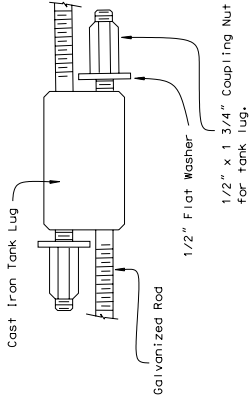


6 Rod Band
Side Elevation

End Section

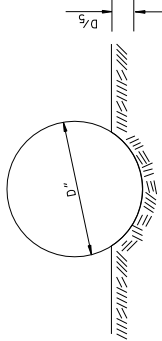
GEOTEXTILE DETAILS FOR JOINTS (BANDS)
(SEE THE SPECIFICATIONS)

COUPLING BAND



TANK LUG

DETAILS COUPLING BAND



MINIMUM PIPE BEDDING REQUIREMENTS
See The Specifications

GENERAL NOTES:

- (1) Type I (helical corrugations) pipe shall have ends reformed to annular corrugations. The reformed annular corrugations shall have the same pitch and depth as the helical corrugations.
- (2) The band shall be of such dimensions that an equal number of corrugations is on each side of the pipe joint.
- (3) The material and coating of the band shall be the same as that of the pipe.
- (4) The gage of the connecting band shall be one gage less than that of the pipe or a minimum of 16 ga.
- (5) The connecting band shall have a minimum circumferential lap of 6".
- (6) All metal hardware (tank rods, nuts, bolts, washers, lugs etc.) shall be galvanized and conform to Material Specification 581 and 582.

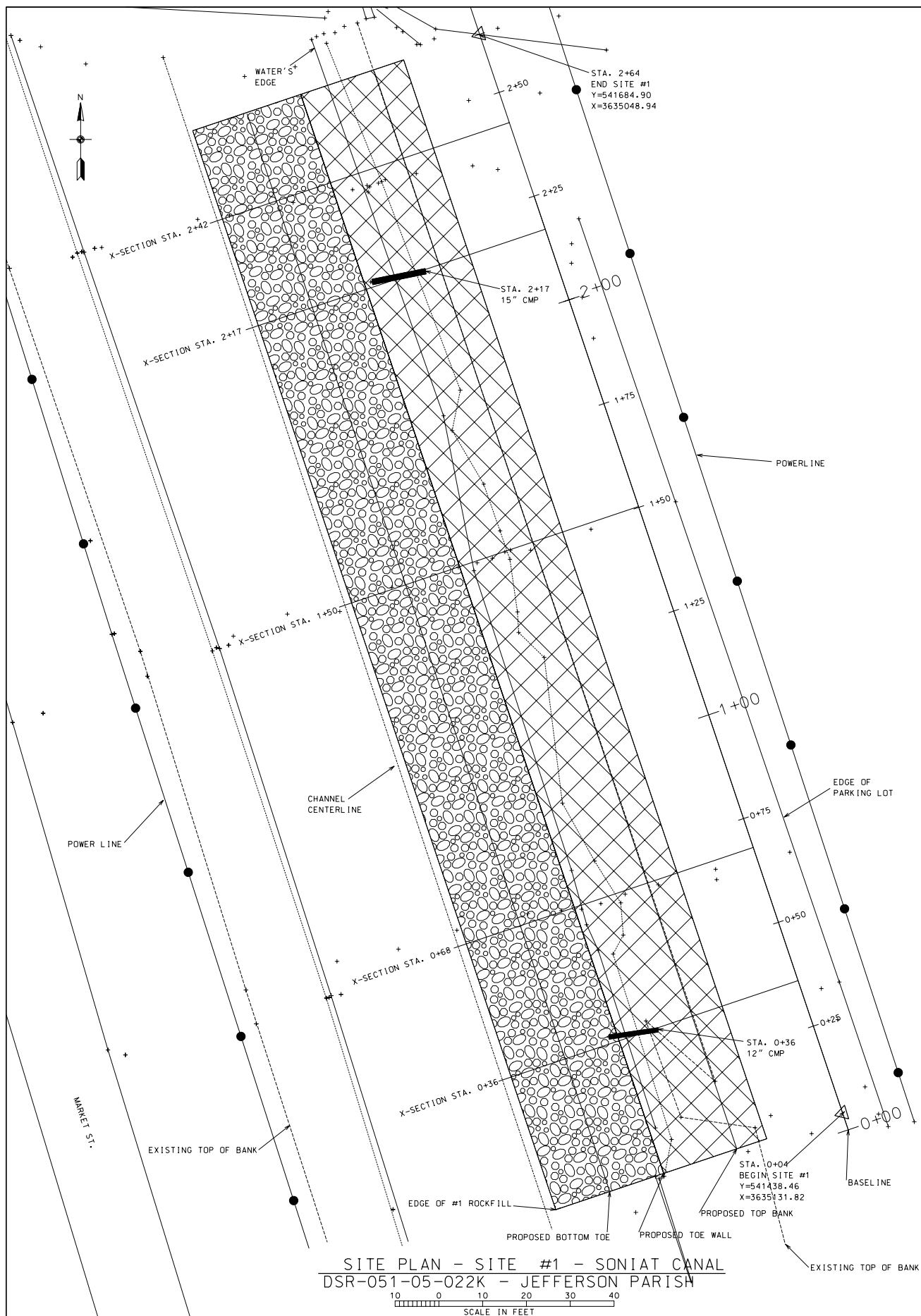


FILE NAME	LA051-05-022K-06
DRAWING NAME	LA051-05-022K-06

NO.	DATE	APPROVED	TITLE

DESIGNED	B. STICKER
DRAWN	ABBEY GREMILLION
CHECKED	CHERIE LAFLEUR
APPROVED	ED CIERING III
DATE	1/07
	1/07
	1/07
	1/07

PIPE DETAILS
EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA



SITE PLAN - SITE #1 - SONIAT CANAL
DSR-051-05-022K - JEFFERSON PARISH

SITE PLAN - SITE #1

EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

DESIGNED	DATE
JIMMY MOORE	03/07
DRAWN	DATE
ABBEY GREMILLION	04/07
CHECKED	DATE
CHERIE LAFLEUR	04/07
APPROVED	DATE
ED GIERING III	04/07



Sheet 07 of 34
FILE NAME
LA051-05-022K-07
DRAWING NAME
LA051-05-022K-07

DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE #1 SECTIONS - STA. 0+36 & 0+68

EWP - HURRICANE KATRINA

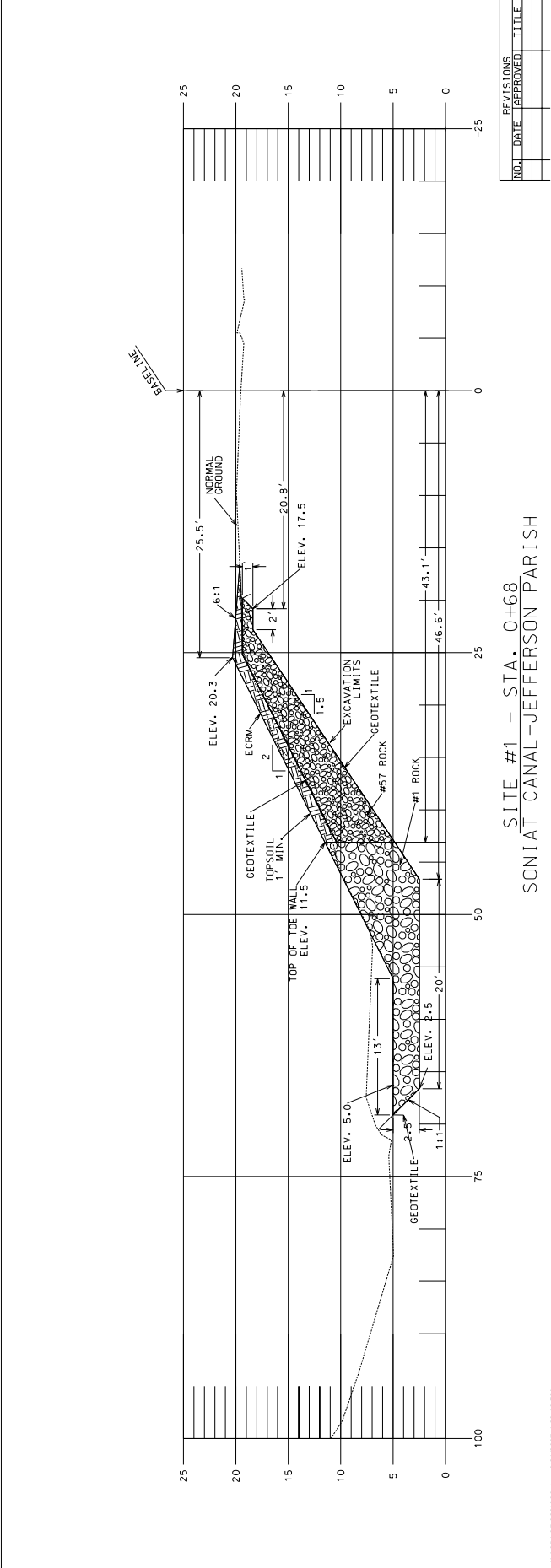
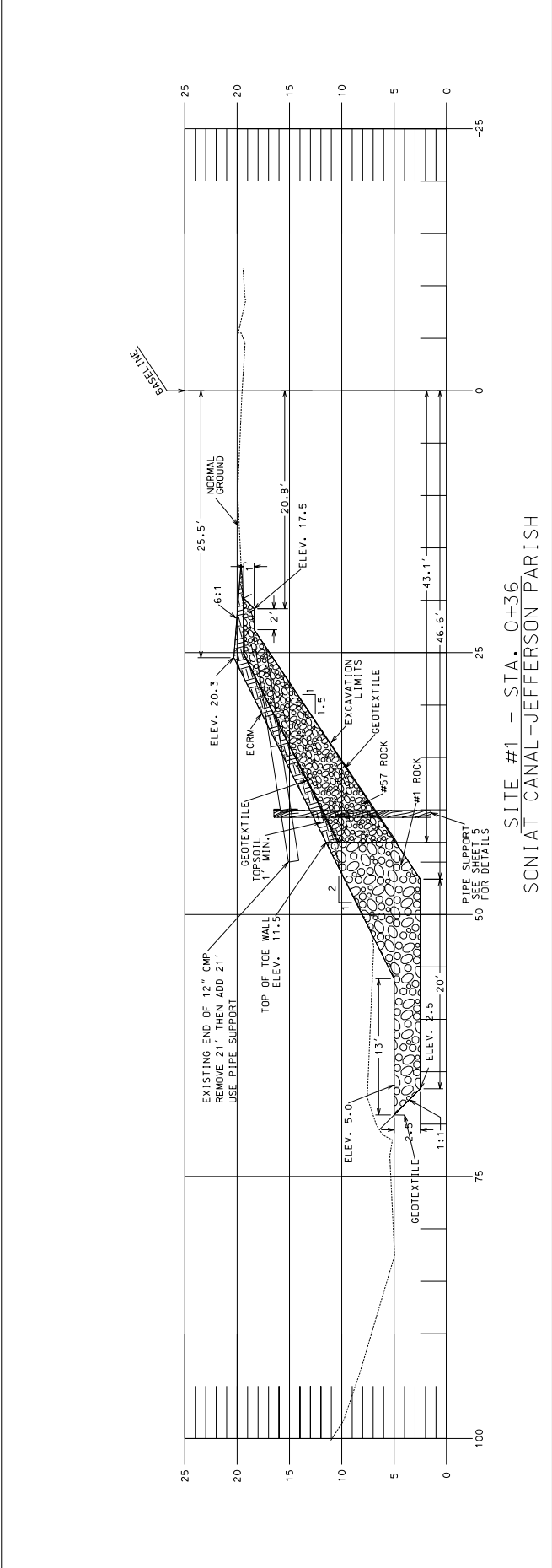
DSR051-05-022K - SONIAT CANAL

JEFFERSON PARISH, LOUISIANA



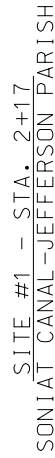
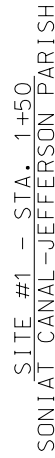
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DRAWING NAME	LA051-05-022K-08

Sheet 08 of 34



NRCS
Natural Resources Conservation Service
United States Department of Agriculture

DATE	DESIGNED	DRAWN	CHECKED	APPROVED
03/07	JIMMY MOORE	MATTHEW SPRINGER	CHERIE LAFLEUR	ED GIERING III
04/07				
04/07				
04/07				



REVISIONS		
NO.	DATE	APPROVED TITLE



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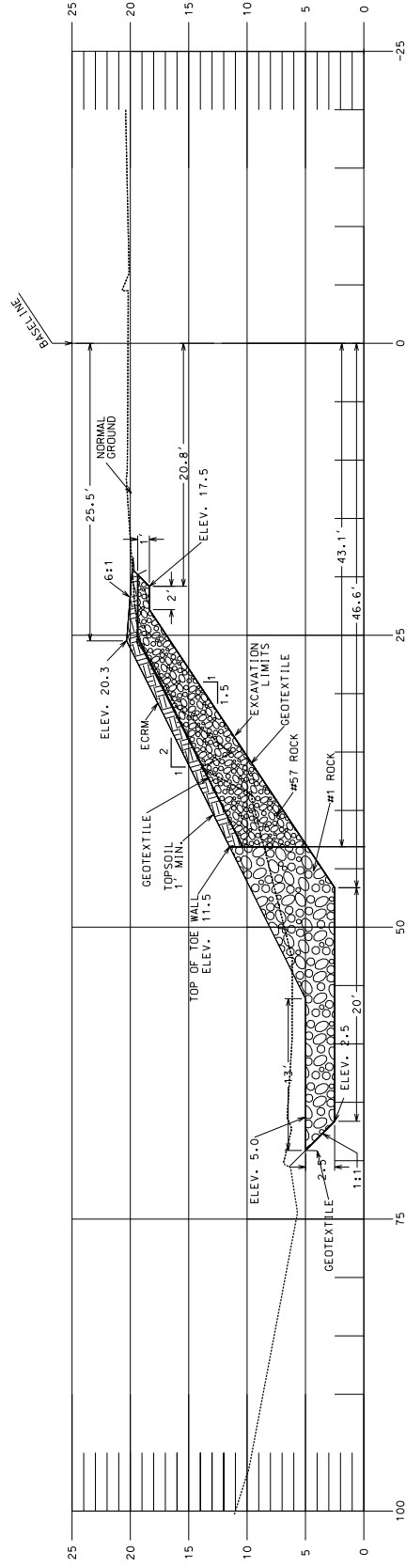
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LA051-05-022K-10

Sheet 10 of 34

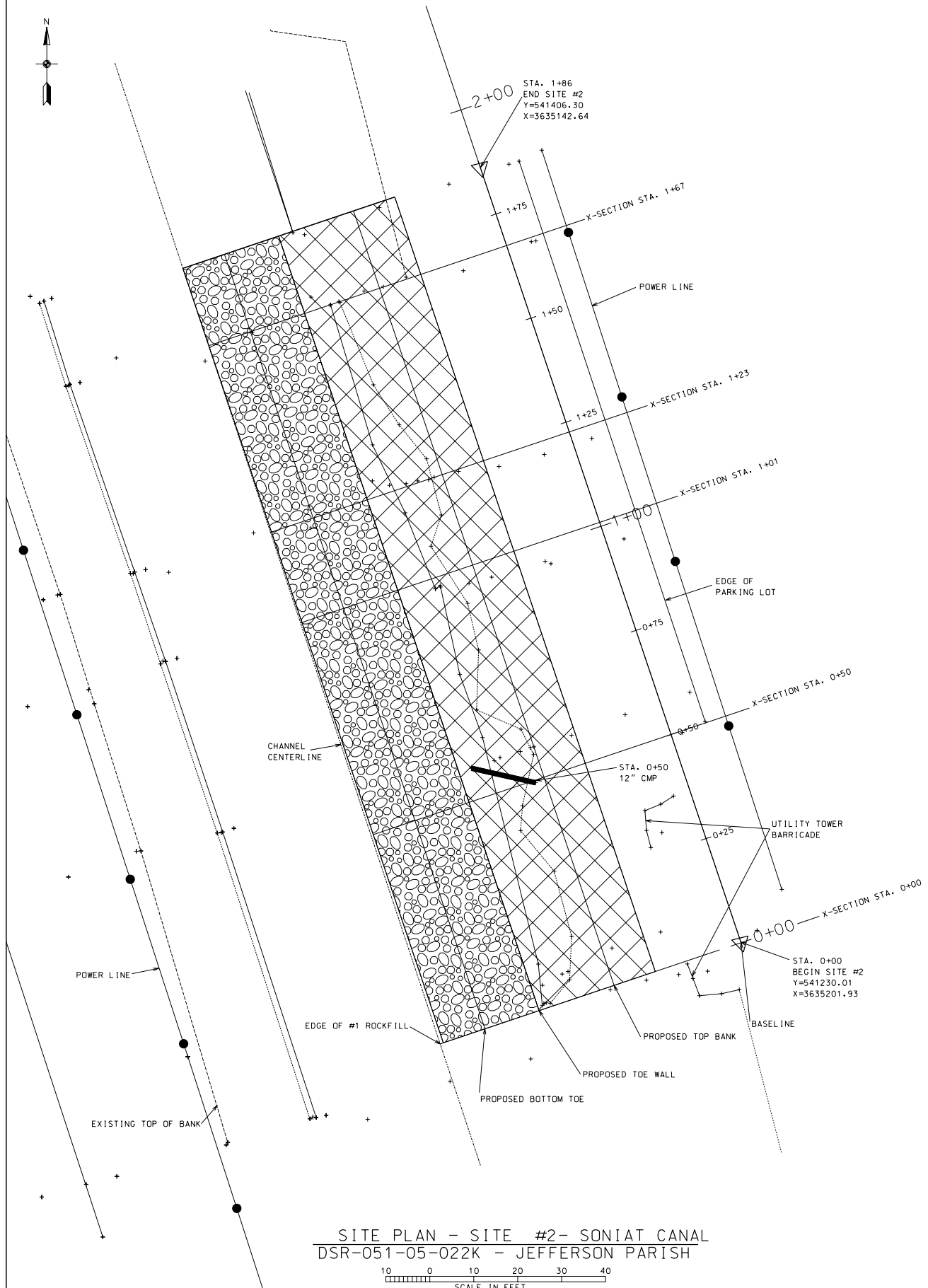
REVISIONS		TITLE	
NO.	DATE	APPROVED	TITLE

SITE #1 SECTION - STA. 2+42
EWP - HURRICANE KATRINA
DSR051-05-022K - SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III
DATE	04/07



SITE #1 - STA. 2+42
SONIAT CANAL-JEFFERSON PARISH



SITE PLAN - SITE #2- SONIAT CANAL
DSR-051-05-022K - JEFFERSON PARISH

10 0 10 20 30 40
SCALE IN FEET



SITE PLAN - SITE #2
EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

	DATE
DESIGNED JIMMY MOORE	03/07
DRAWN ABBEY GREMILLION	04/07
CHECKED CHERIE LAFLEUR	04/07
APPROVED ED GIERING III	04/07



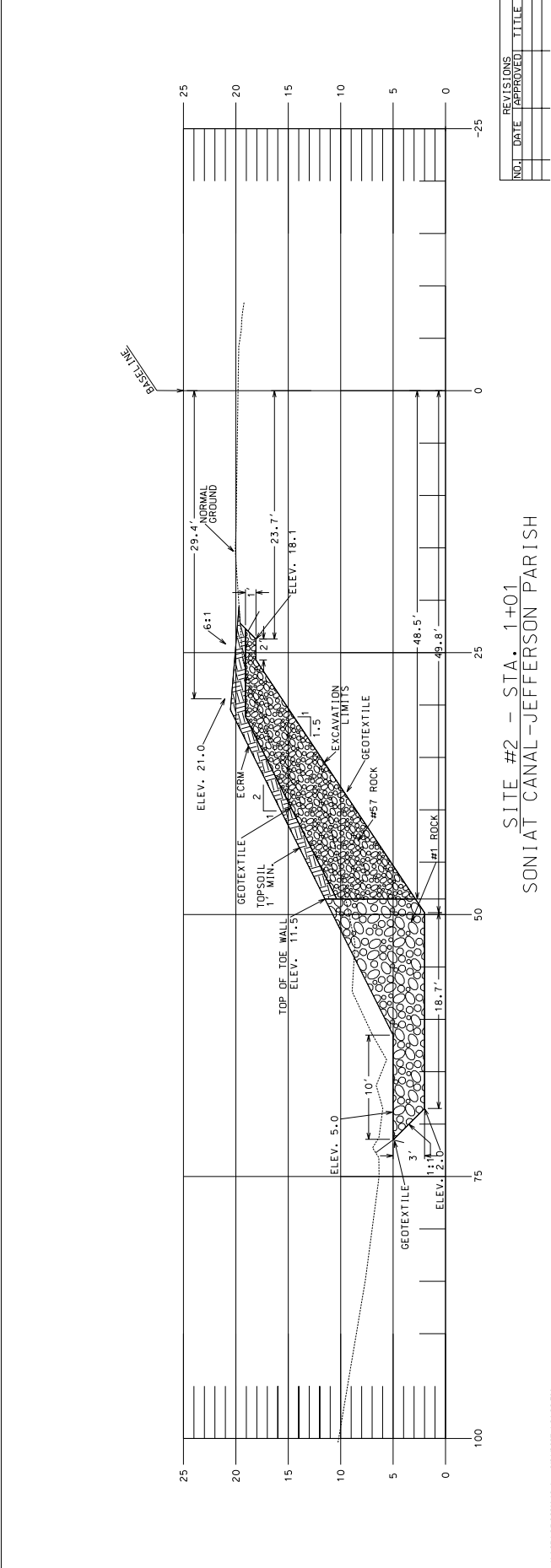
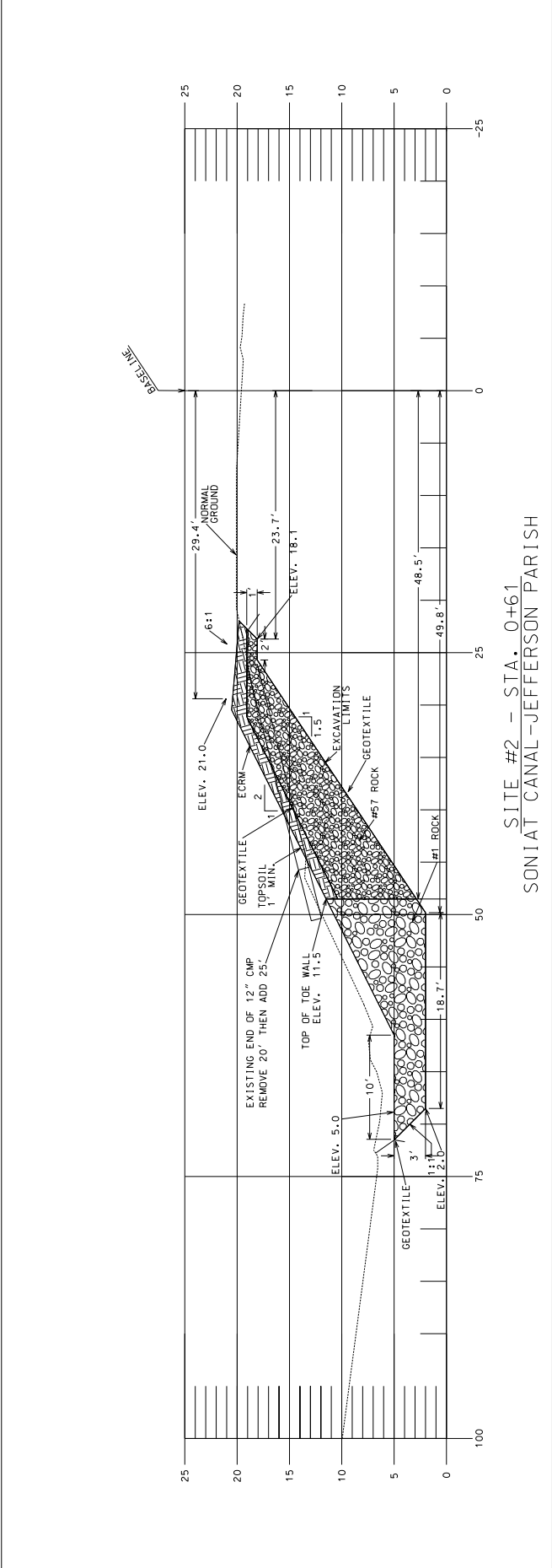
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SONIAT CANAL-JEFFERSON PARISH

DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE #2 SECTIONS - STA. 0+61 & 1+01
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-13
DRAWING NAME	LA051-05-022K-13



SITE #2 - STA. 1+01
 SONIAT CANAL-JEFFERSON PARISH

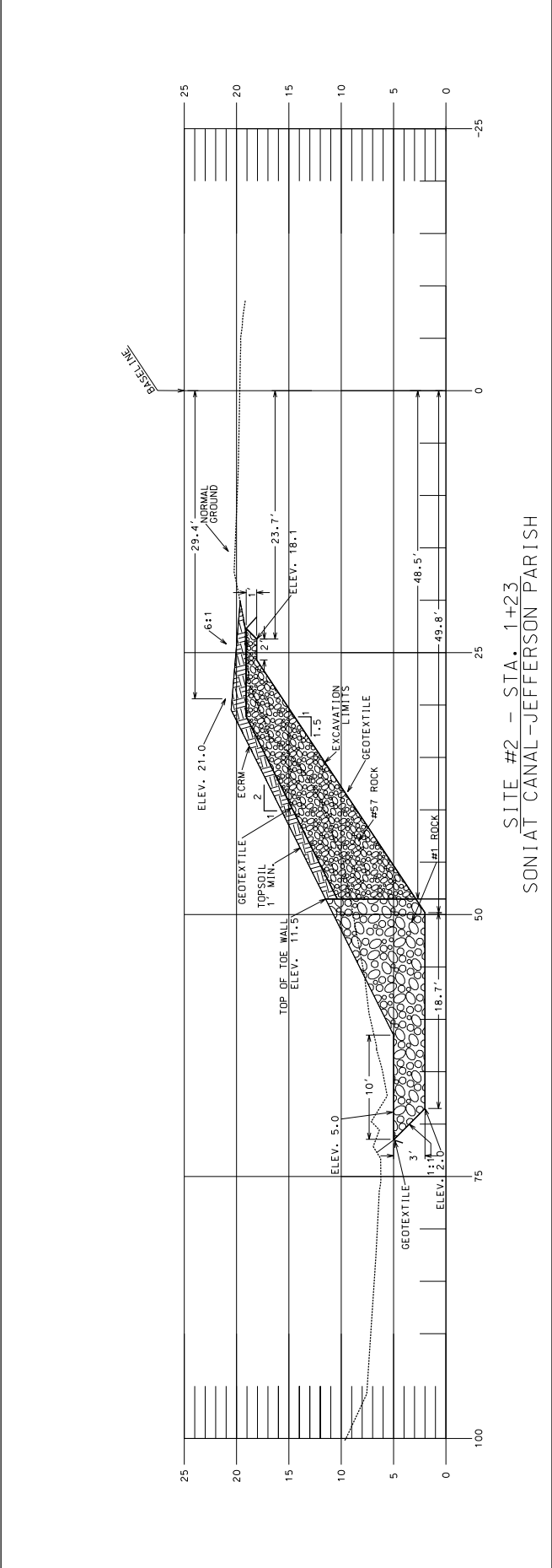
NO.	DATE	APPROVED	TITLE

DATE	03/07
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

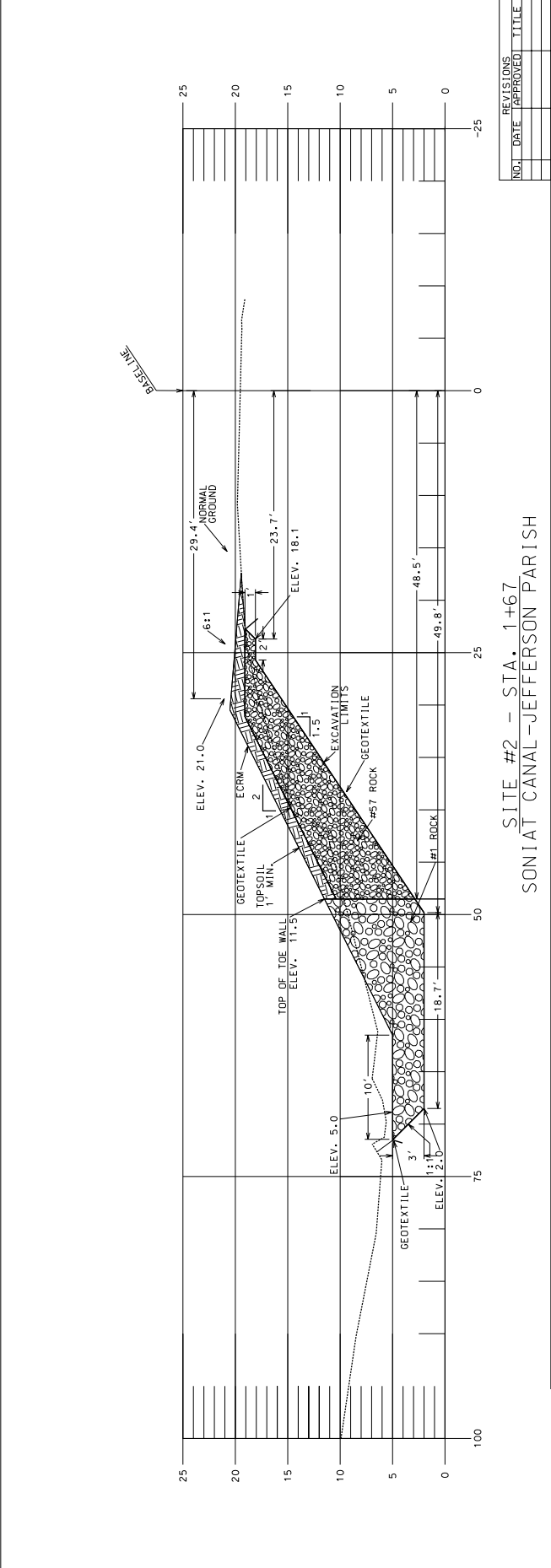
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 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



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DRAWING NAME	LA051-05-022K-14

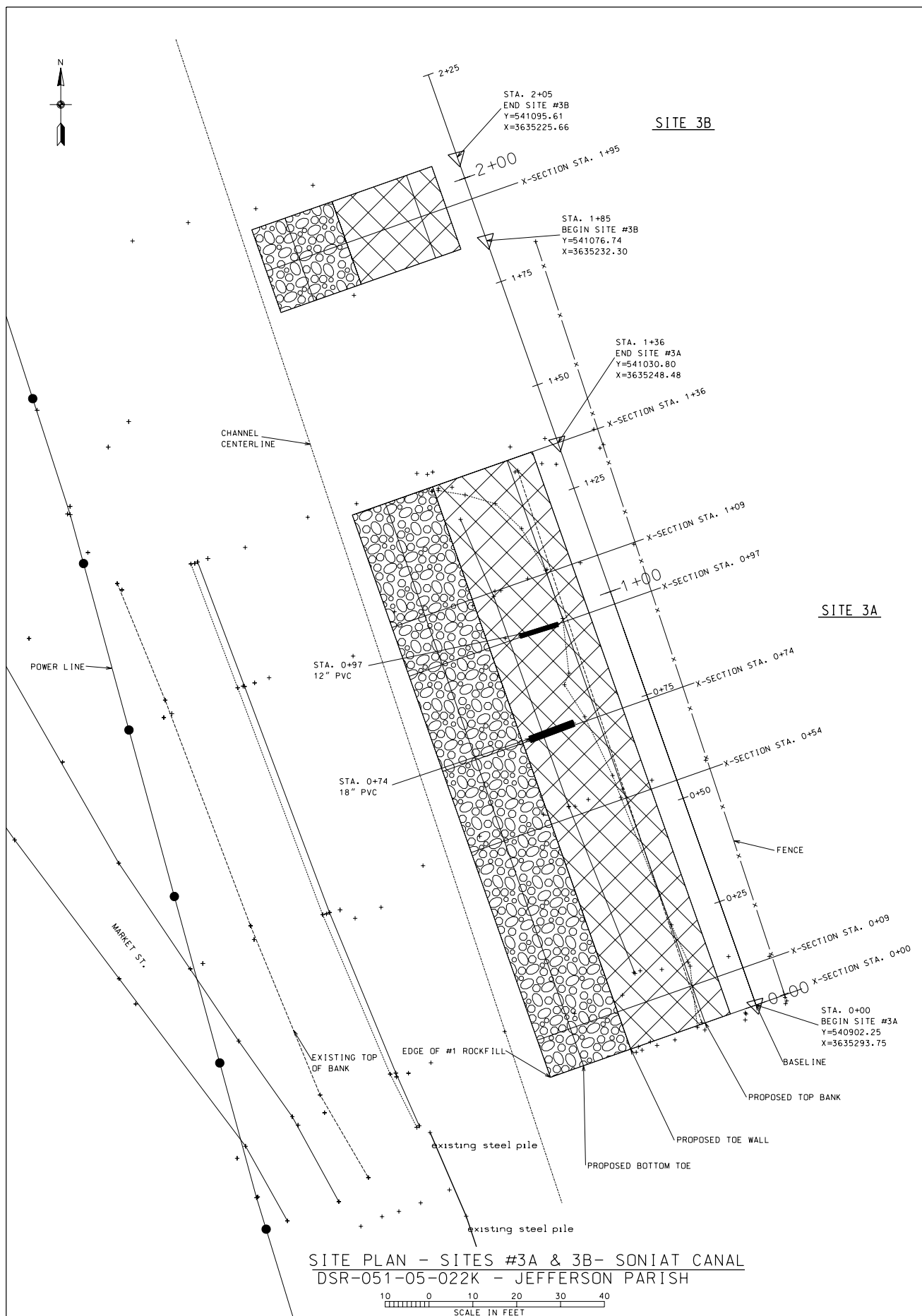


SITE #2 - STA. 1+23
 SONIAT CANAL-JEFFERSON PARISH



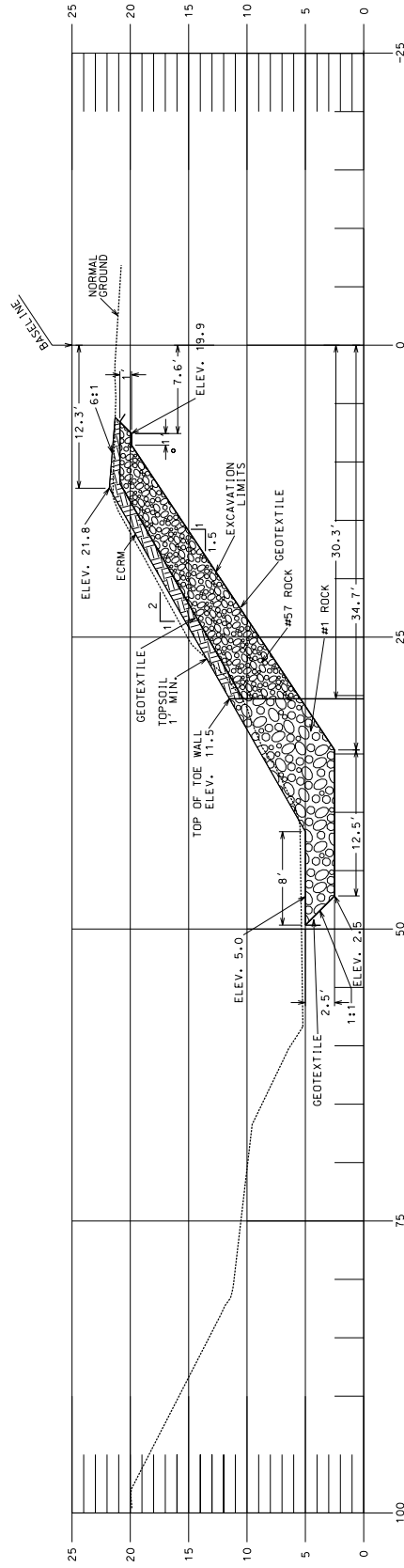
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 SONIAT CANAL-JEFFERSON PARISH

NO.	DATE	APPROVED	TITLE

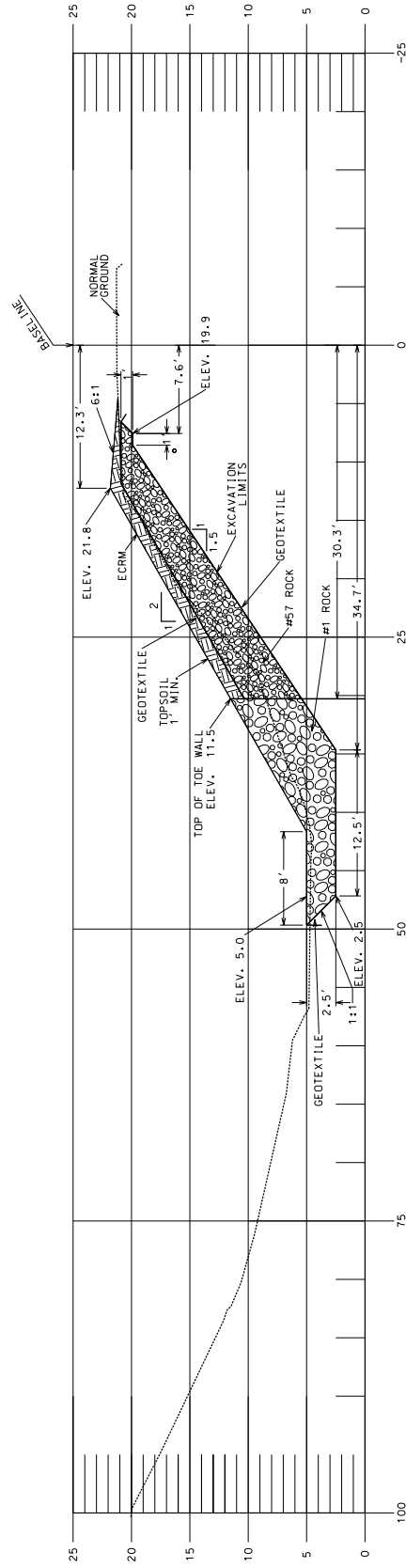


SITE #3A SECTIONS - STA. 0+00 & 0+09
EWP - HURRICANE KATRINA
DSR051-05-022K - SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

DATE	DESIGNED	JIMMY MOORE	03/07
04/07	BRAIN	MATTHEW SPRINGER	04/07
04/07	CHECKED	CHERIE LAFLEUR	04/07
04/07	APPROVED	ED GIERING III	04/07



SITE #3A - STA. 0+00
SONIAT CANAL-JEFFERSON PARISH



SITE #3A - STA. 0+09
SONIAT CANAL-JEFFERSON PARISH

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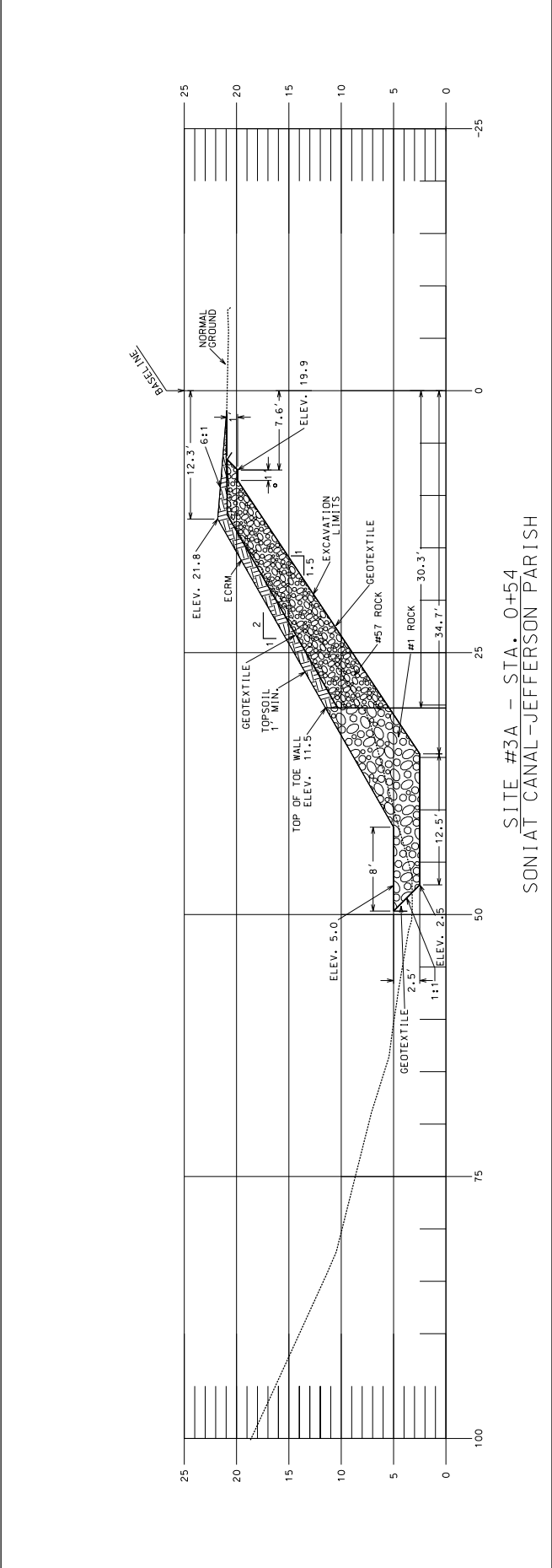
DATE	03/07
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

DESIGNED JIMMY MOORE
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA

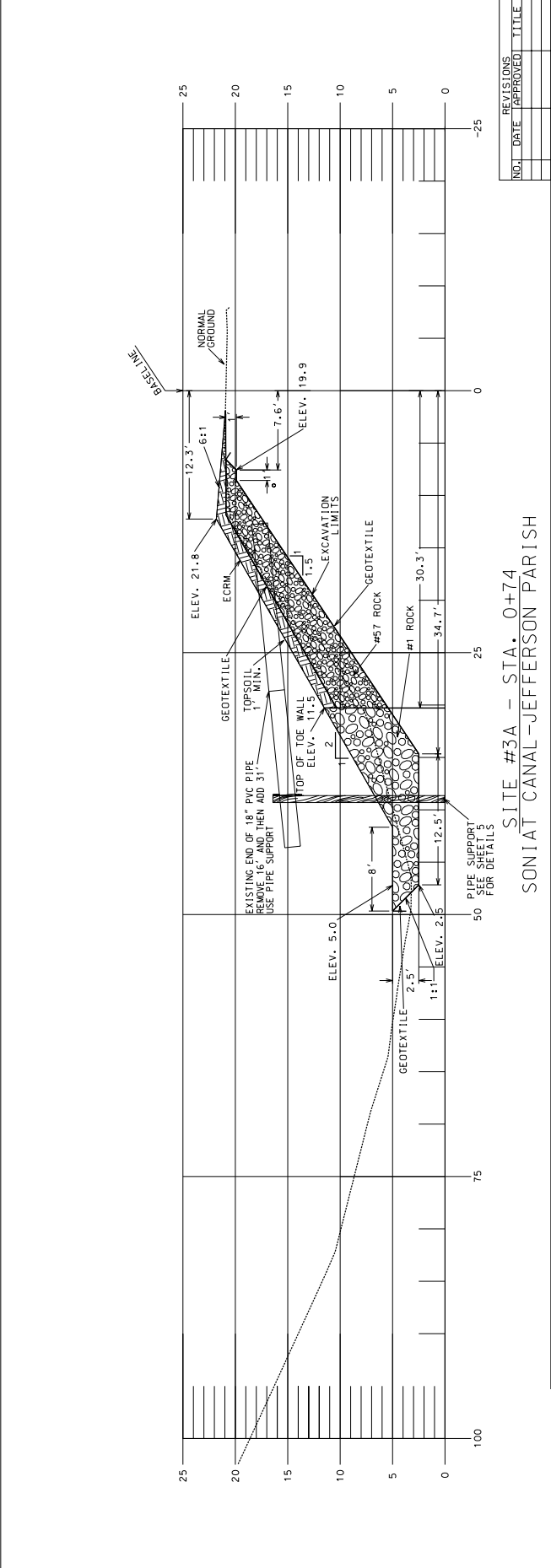


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DRAWING NAME	LA051-05-022K-17

NO.	DATE	APPROVED	TITLE



SITE #3A - STA. 0+54
 SONIAT CANAL-JEFFERSON PARISH



SITE #3A - STA. 0+74
 SONIAT CANAL-JEFFERSON PARISH

DATE	03/07
DRAWN	JIMMY MOORE
CHECKED	MATTHEW SPRINGER
APPROVED	ED GIERING, III

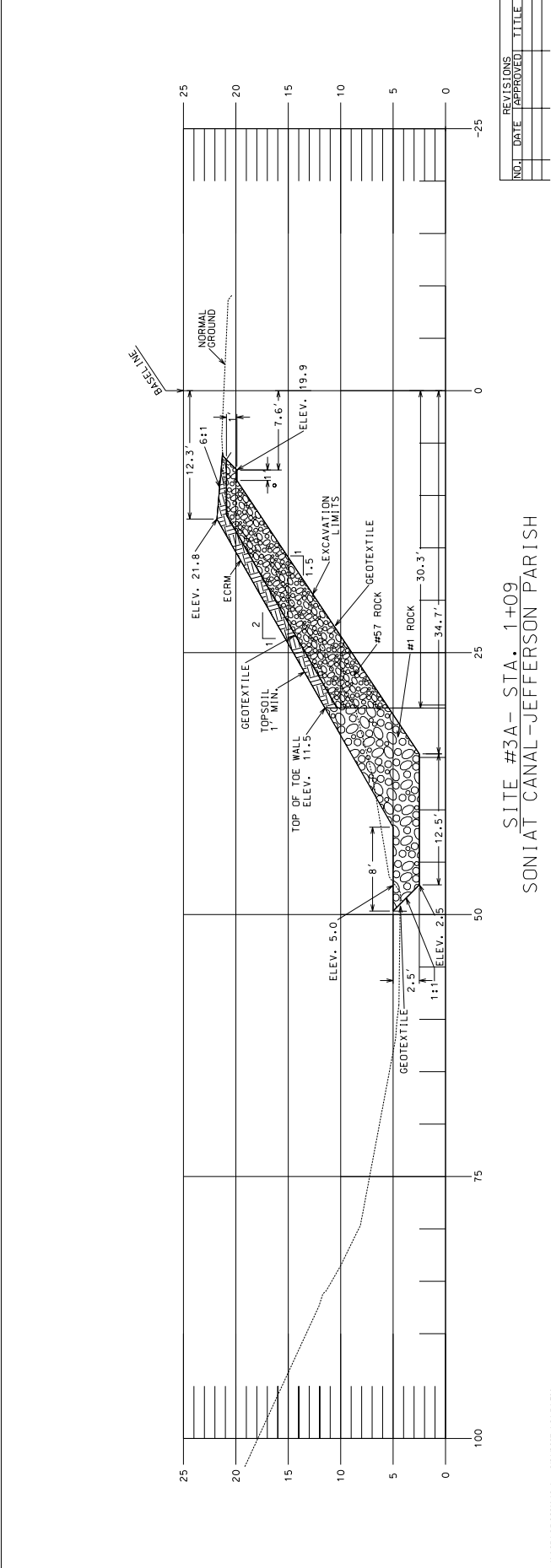
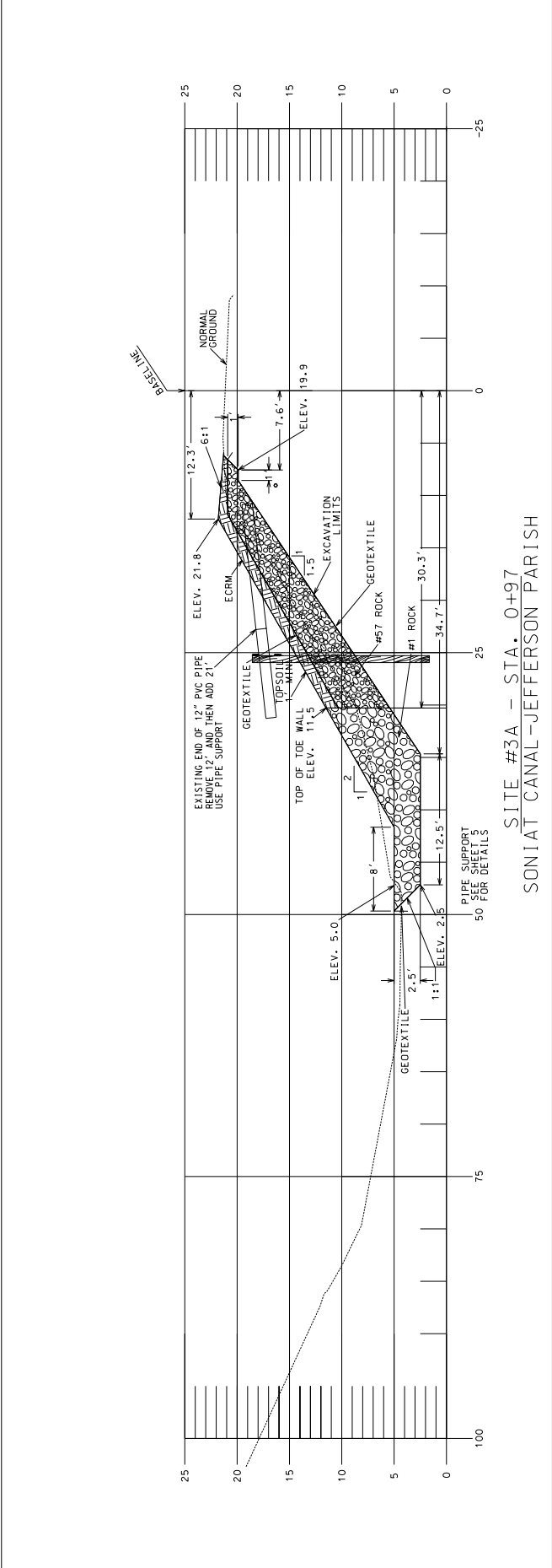
SITE #3A SECTIONS - STA. 0+95 & 1+09
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-18
DRAWING NAME	LA051-05-022K-18

Sheet 18 of 34

NO.	DATE	APPROVED	TITLE

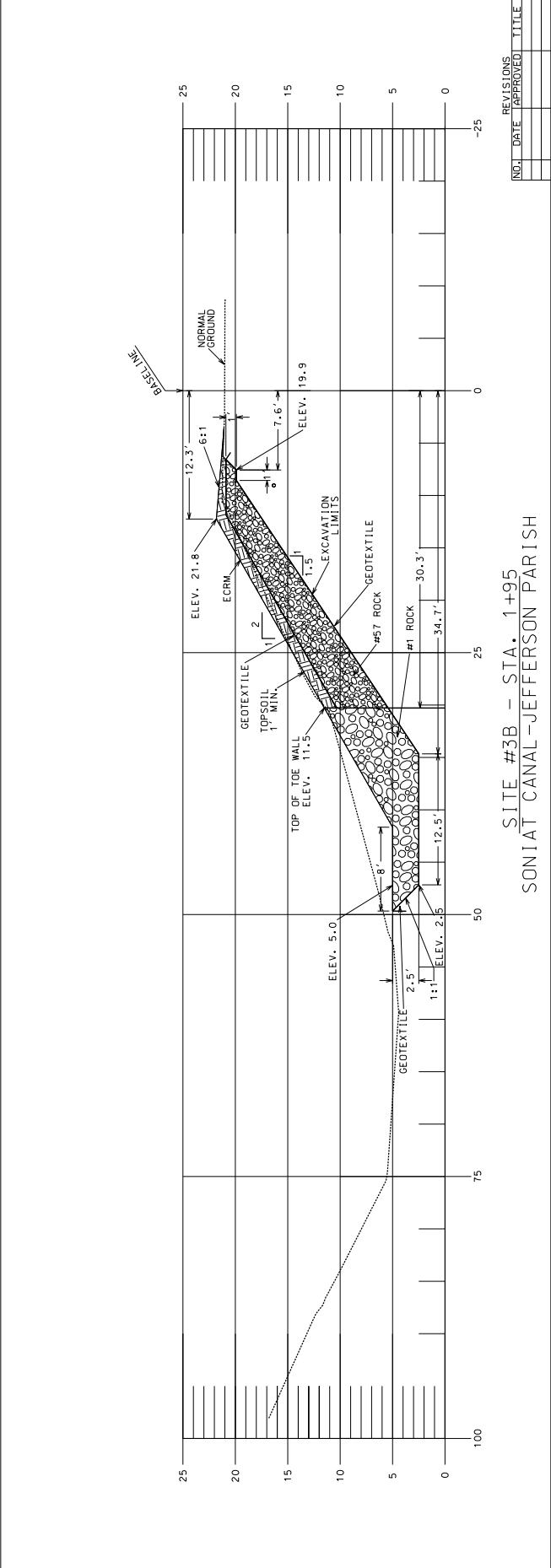
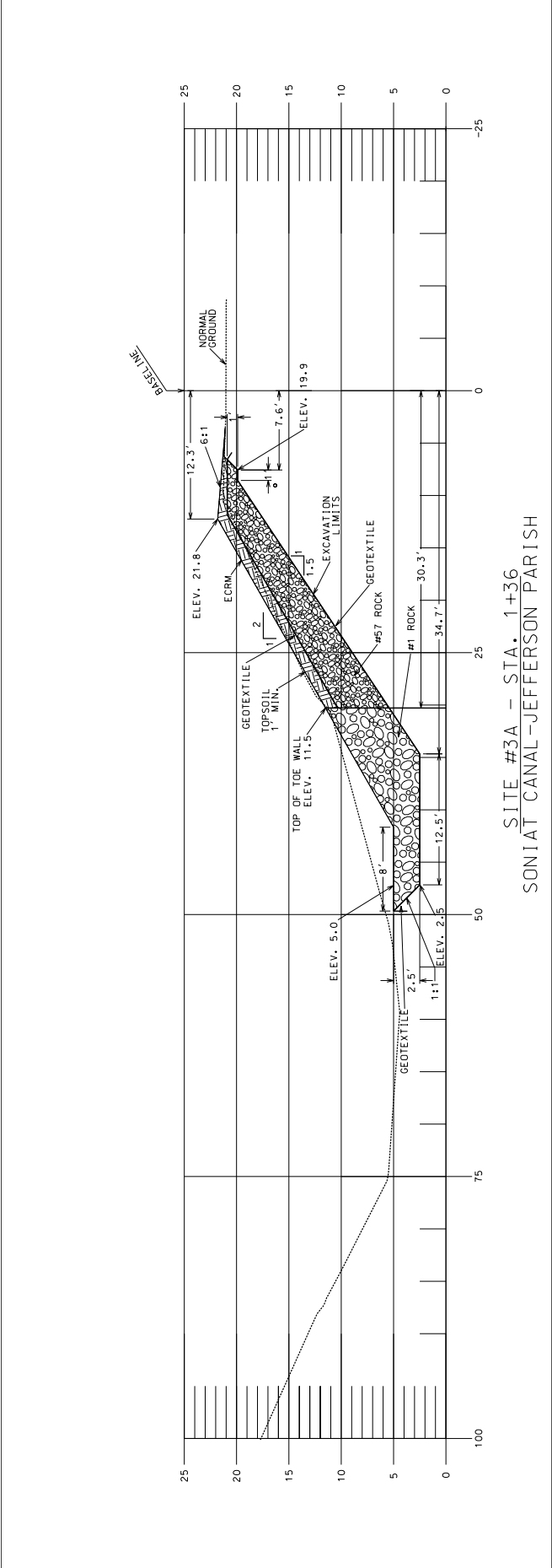


DATE	03/07
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED CIERING, III

DESIGNED JIMMY MOORE
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA
 SITE #3A/B SECTIONS - STA. 1+36 & 1+95



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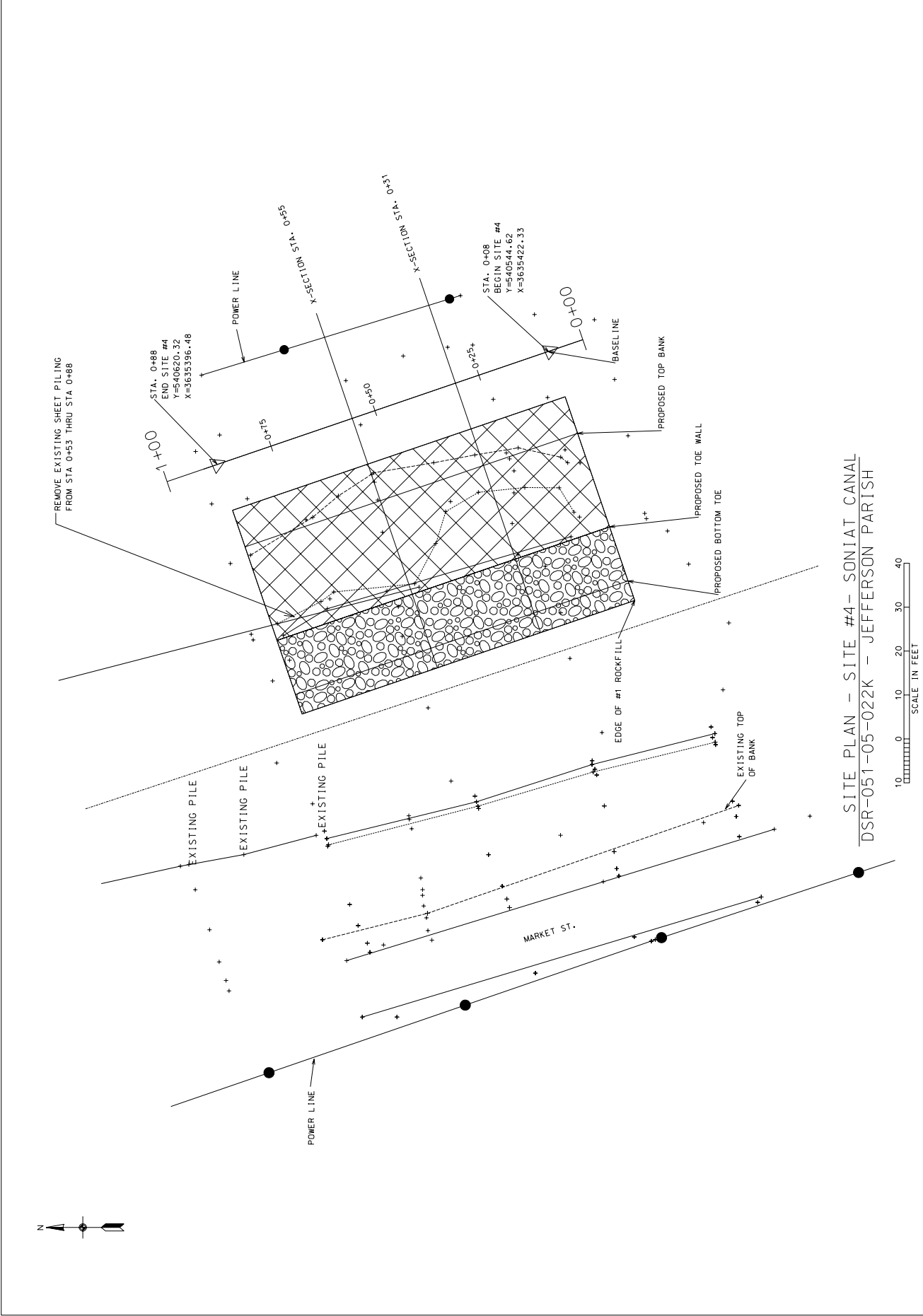
NO.	DATE	APPROVED	TITLE

DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	ABBEY GREMILLION
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE PLAN - SITE # 4
 EWP - HURRICANE KATRINA
 DSR-051-05-022K-SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-20
DRAWING NAME	LA051-05-022K-20



SITE PLAN - SITE #4- SONIAT CANAL
 DSR-051-05-022K - JEFFERSON PARISH

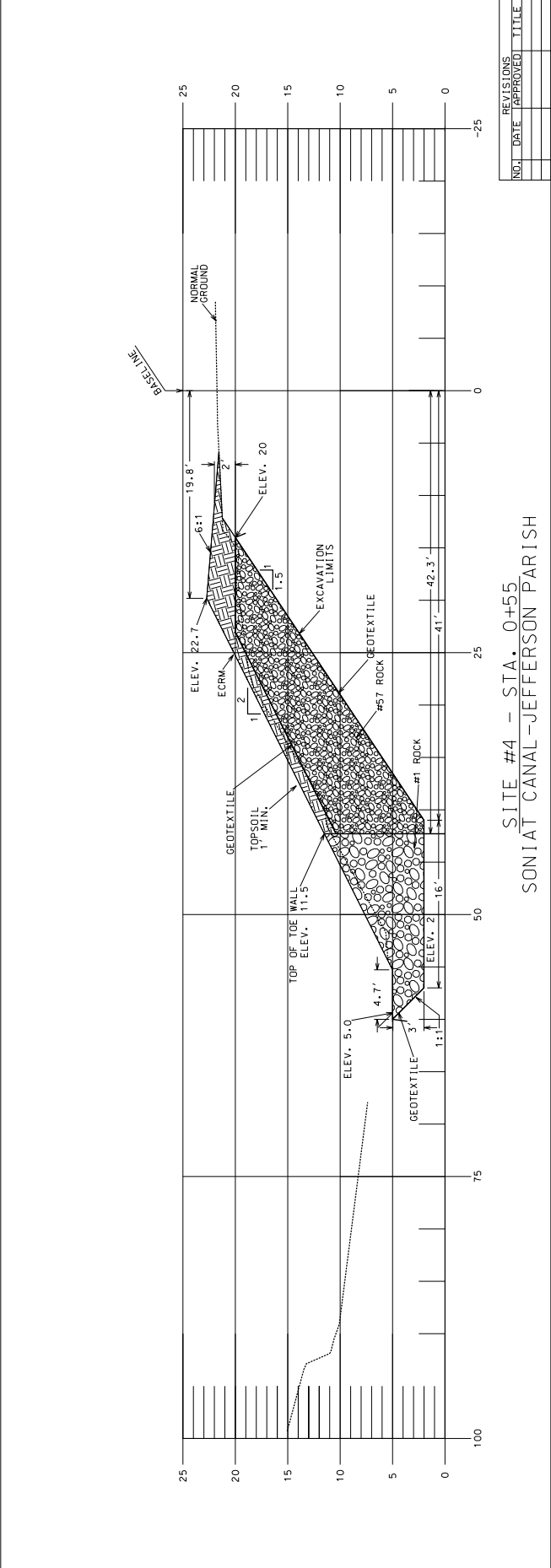
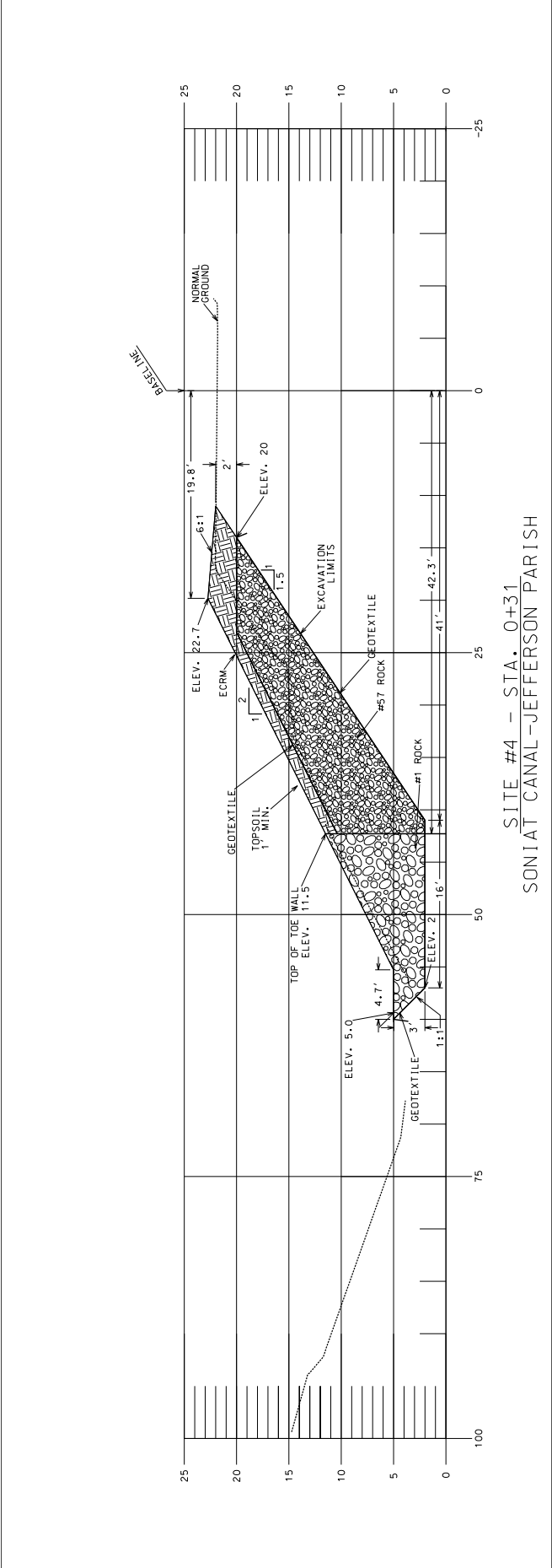
DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE #4 SECTIONS - STA. 0+31 & 0+55
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



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NO.	DATE	APPROVED	TITLE

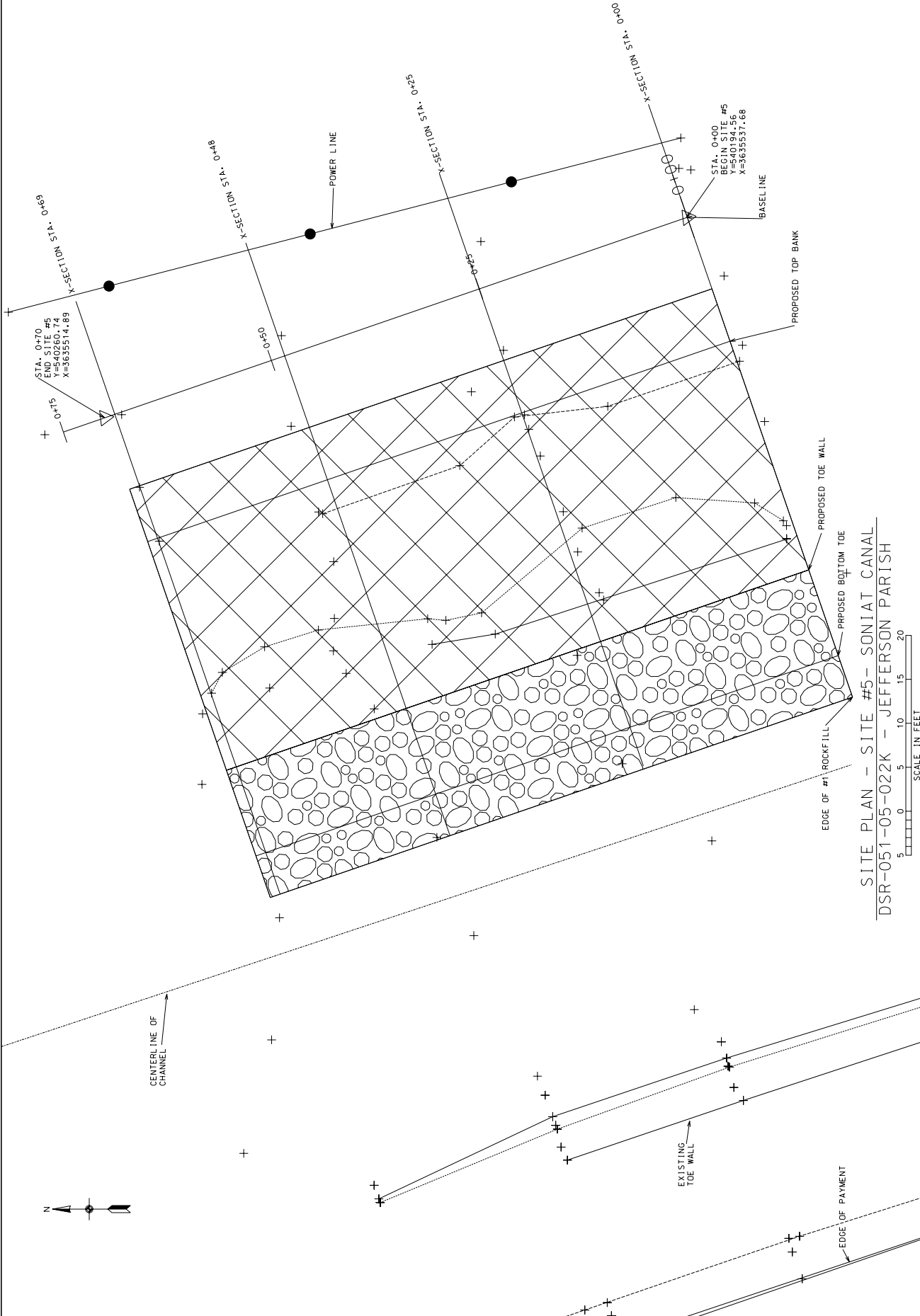


DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	ABBIE GREMILLION
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE PLAN - SITE # 5
 EWP - HURRICANE KATRINA
 DSR-051-05-022K-SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



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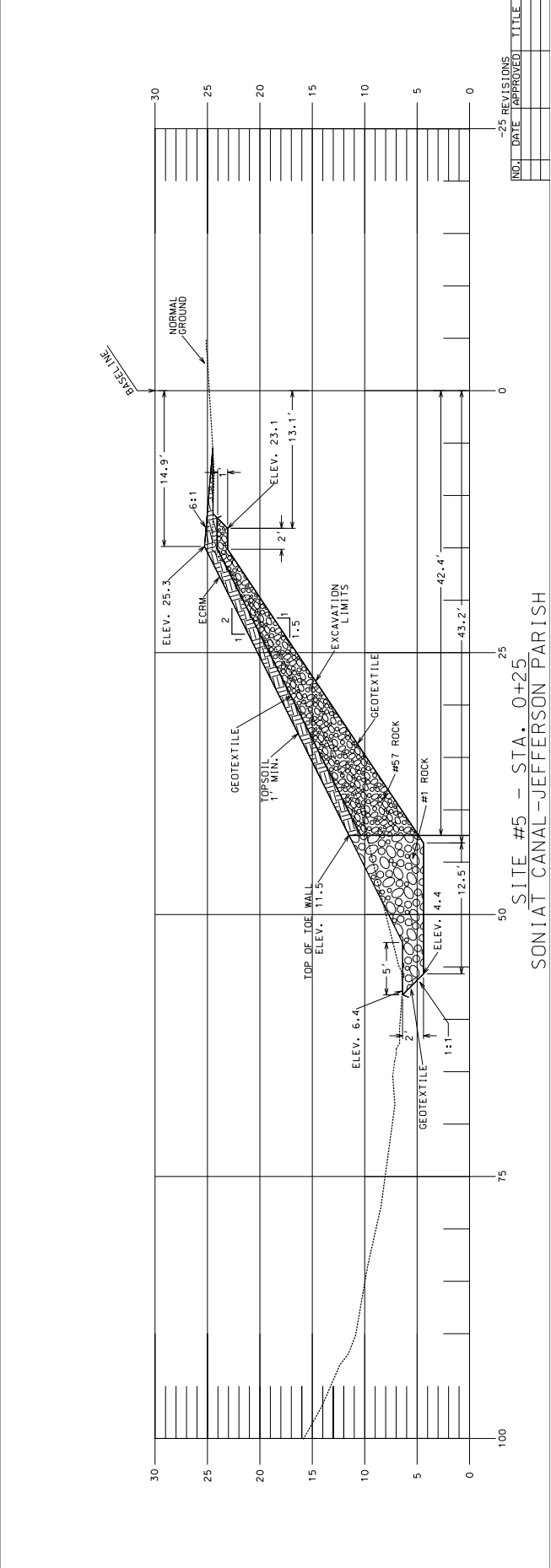
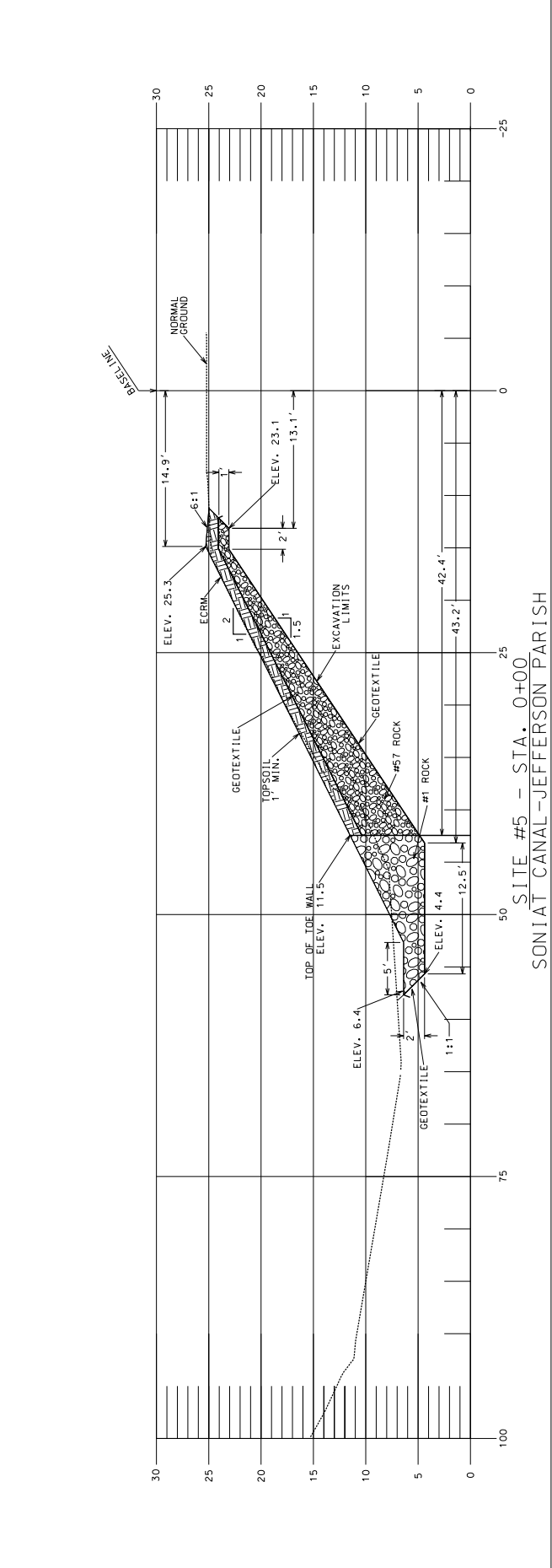
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 DSR-051-05-022K - JEFFERSON PARISH

DATE	03/07
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

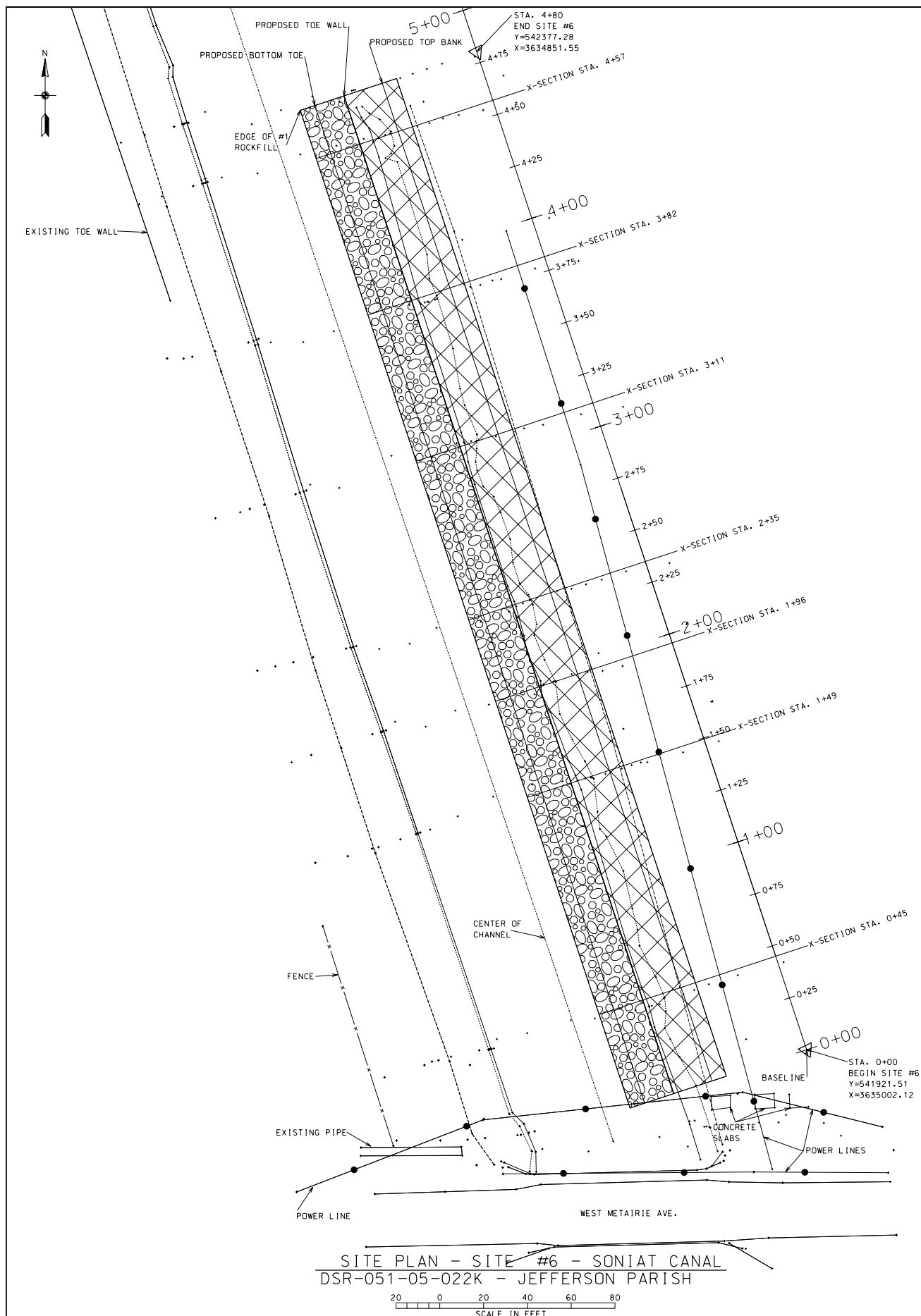
SITE #5 SECTIONS - STA. 0+00 & 0+25
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-23
DRAWING NAME	LA051-05-022K-23







SITE PLAN - SITE #6 - SONIAT CANAL
DSR-051-05-022K - JEFFERSON PARISH

20 0 20 40 60 80
SCALE IN FEET

SITE PLAN - SITE #6
EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA



	DATE
DESIGNED JIMMY MOORE	03/07
DRAWN ABBEY GREMILLION	04/07
CHECKED CHERIE LAFLEUR	04/07
APPROVED ED GIERING III	04/07

DATE	DESIGNED	JIMMY MOORE
03/07		
04/07	DRAWN	MATTHEW SPRINGER
04/07	CHECKED	CHERIE LAFLEUR
04/07	APPROVED	ED GIERING III



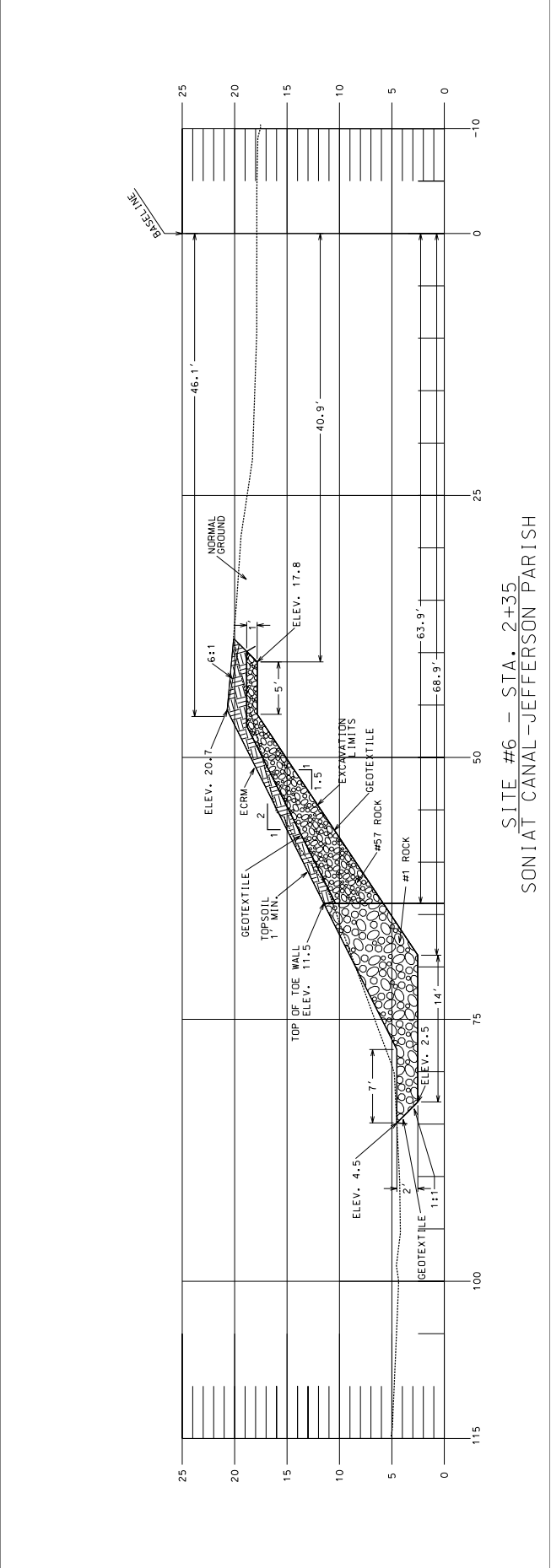
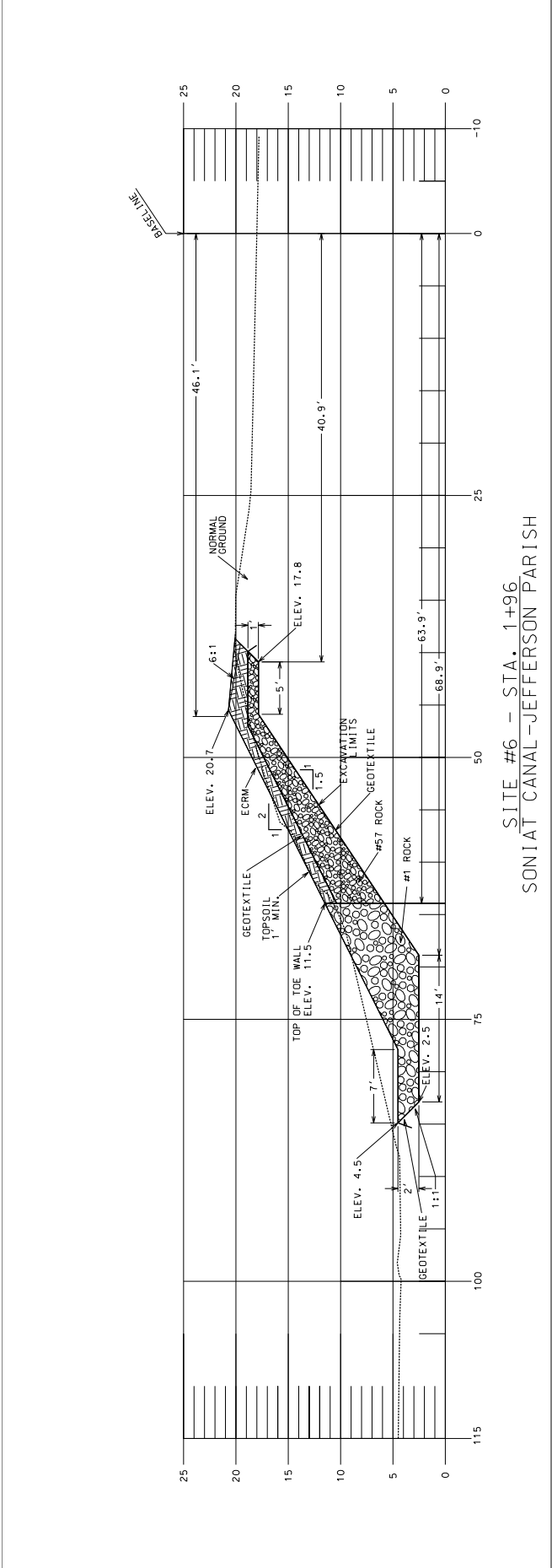
DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED CIERING, III

SITE #6 SECTIONS - STA. 1+96 & 2+35
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-27
DRAWING NAME	LA051-05-022K-27

Sheet 27 of 34



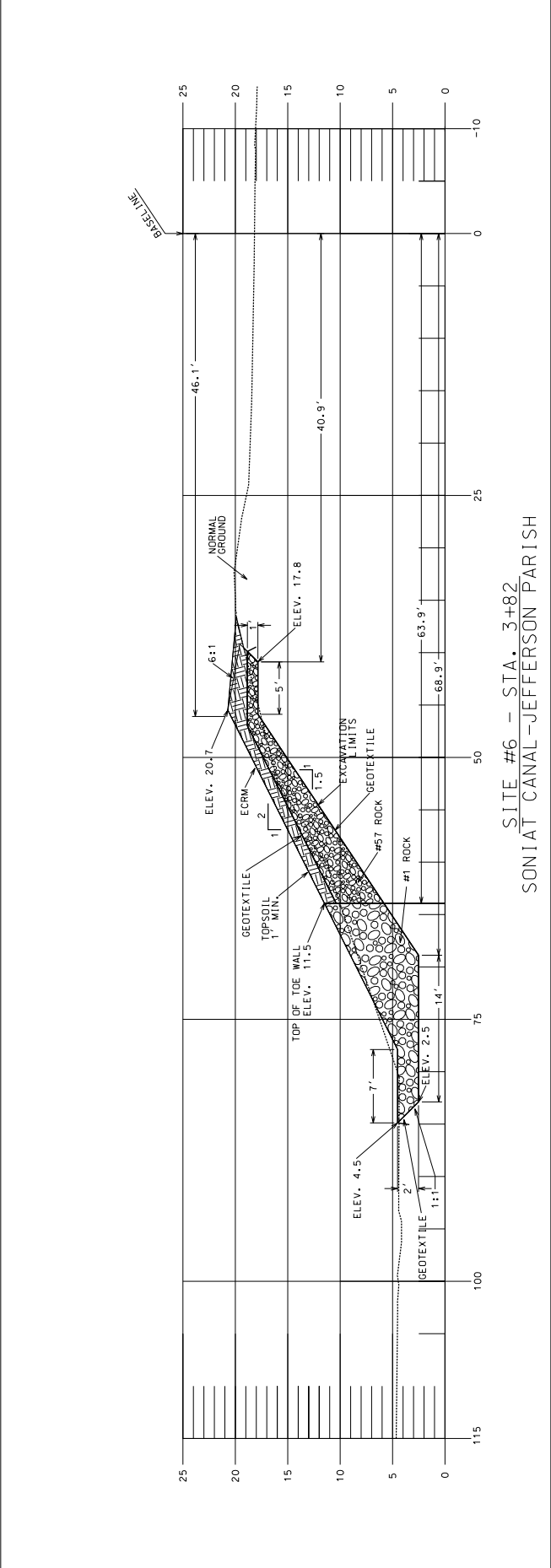
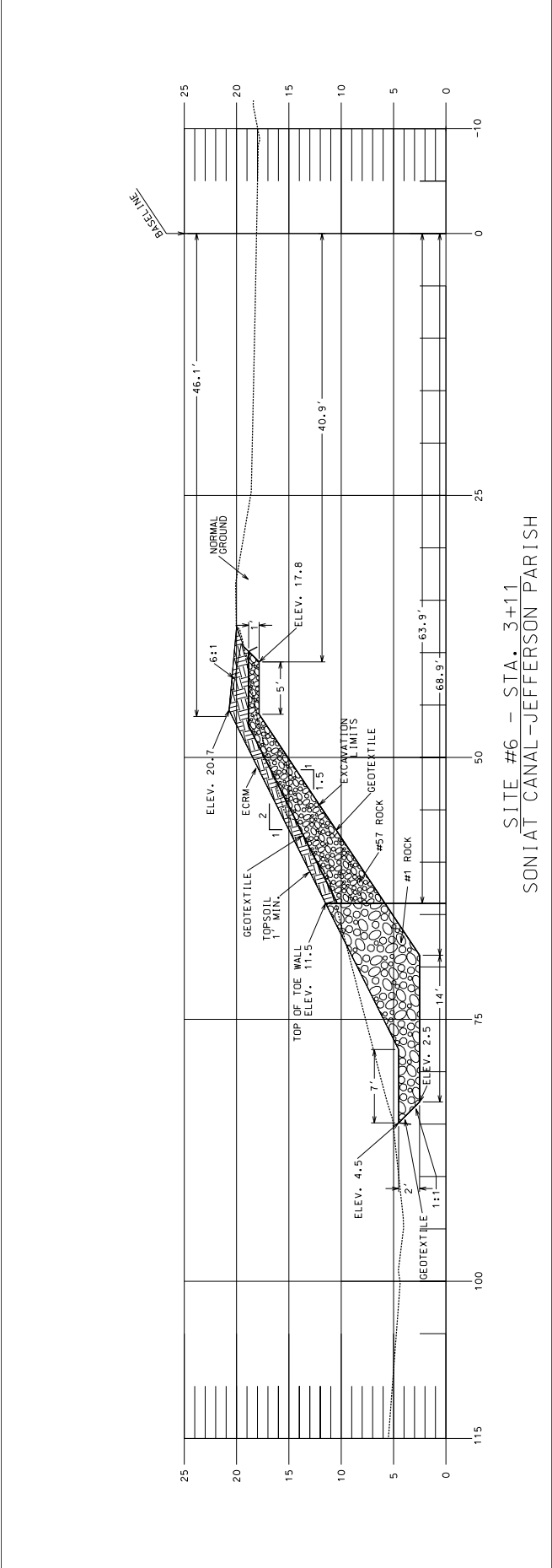
DATE	03/07
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE #6 SECTIONS - STA. 3+11 & 3+82
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



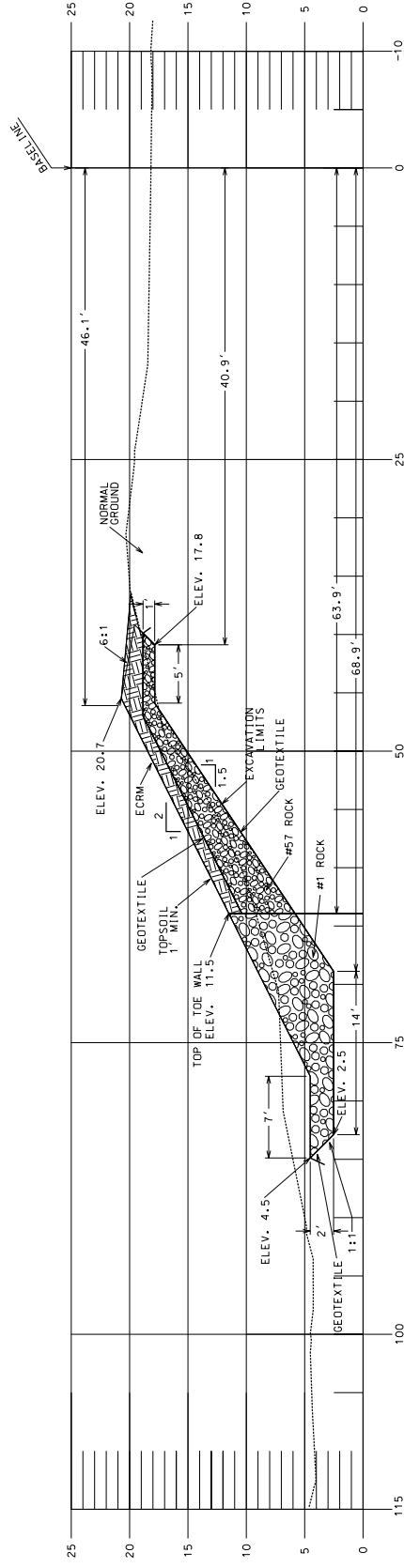
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DRAWING NAME	LA051-05-022K-28

Sheet 28 of 34



SITE #6 SECTION - STA. 4+57
EWP - HURRICANE KATRINA
DSR051-05-022K - SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

DATE	DESIGNED	DRAWN	CHECKED	APPROVED
03/07	JIMMY MOORE	MATTHEW SPRINGER	CHERIE LAFLEUR	ED GIERING III
04/07				
04/07				
04/07				



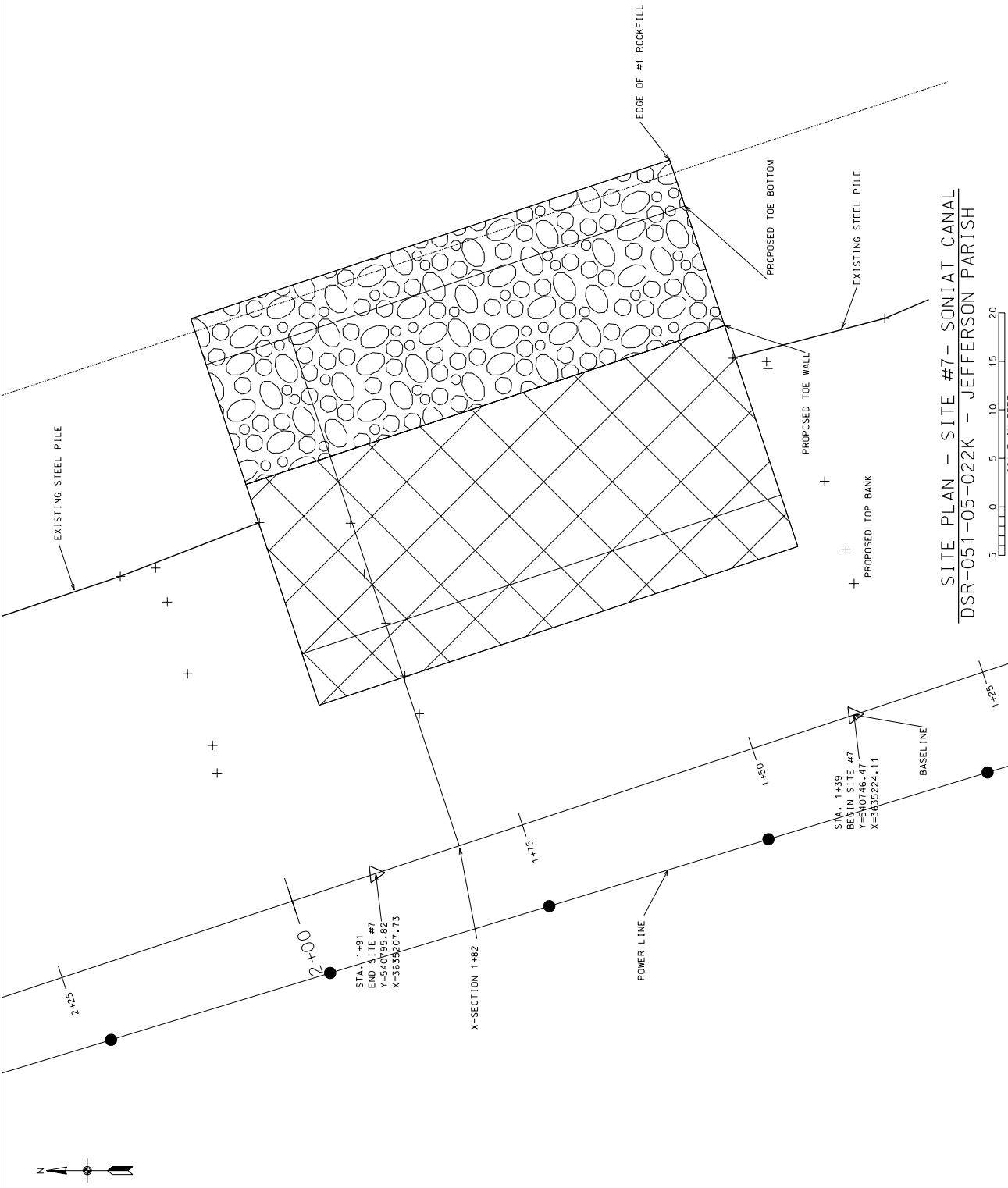
SITE #6 - STA. 4+57
SONIAT CANAL-JEFFERSON PARISH

DESIGNED	JIMMY MOORE	DATE	03/07
DRAWN	ABBIE GREMILLION		04/07
CHECKED	CHERIE LAFLEUR		04/07
APPROVED	ED GIERING, III		04/07

SITE PLAN - SITE # 7
 EWP - HURRICANE KATRINA
 DSR-051-05-022K-SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



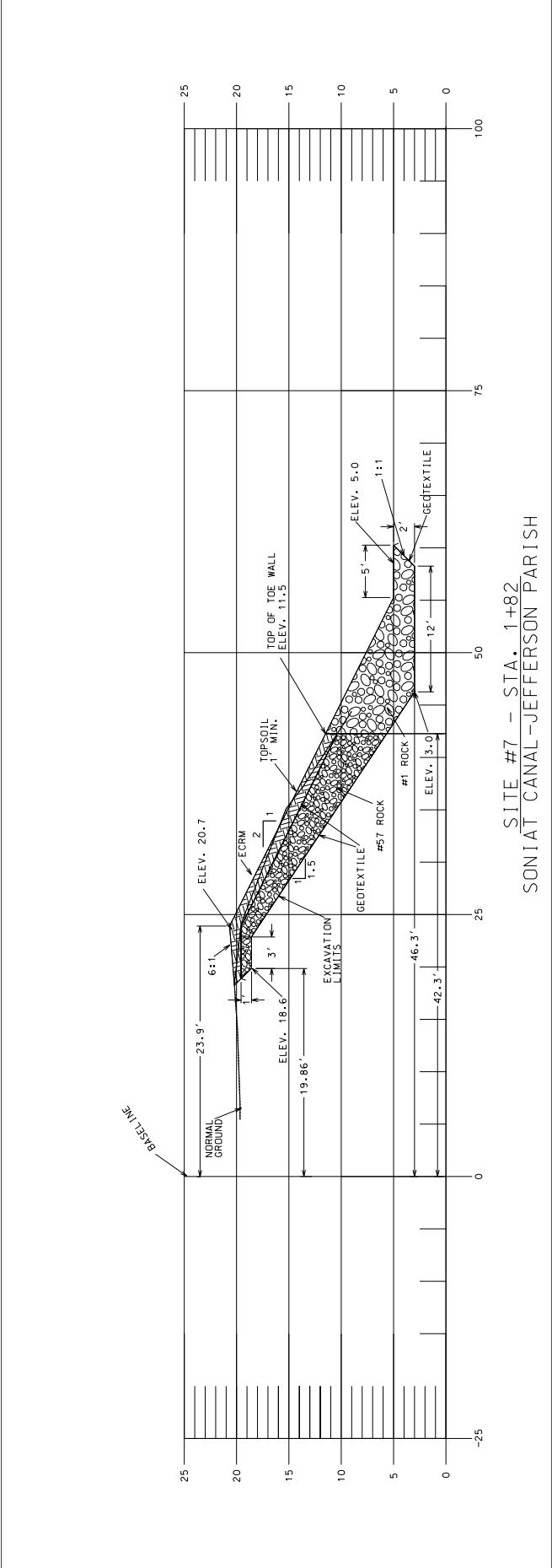
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DRAWING NAME	LA051-05-022K-30



SITE PLAN - SITE #7- SONIAT CANAL
 DSR-051-05-022K - JEFFERSON PARISH



DATE	03/07
DRAWN	JIMMY MOORE
CHECKED	MATTHEW SPRINGER
APPROVED	ED GIERING, III

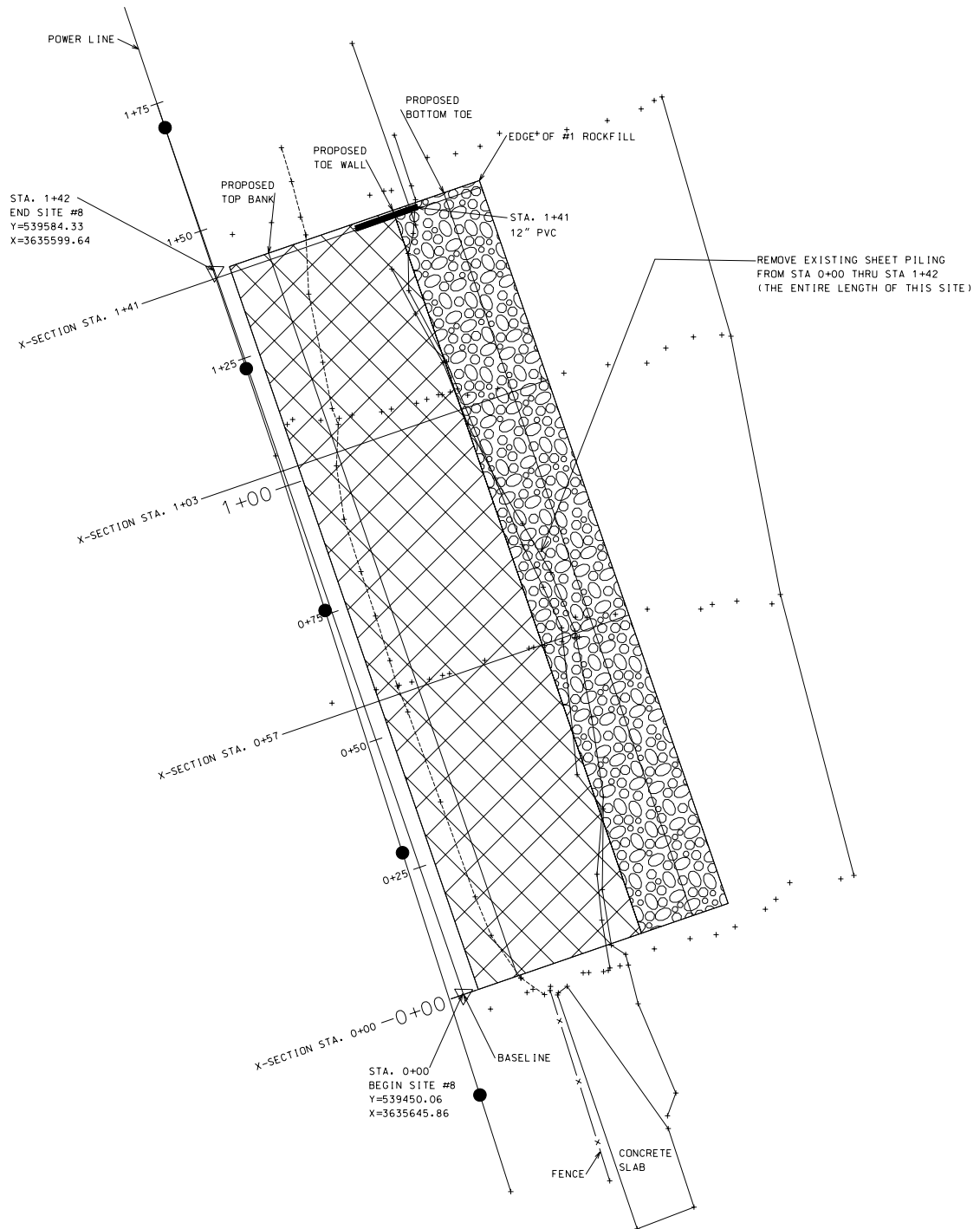
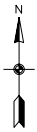


SITE #7 SECTION - STA. 1+82
EWP - HURRICANE KATRINA
DSR051-05-022K - SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

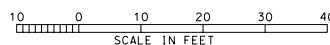


FILE NAME	LA051-05-022K-31
DRAWING NAME	LA051-05-022K-31

NO.	DATE	REVISIONS	TITLE



SITE PLAN - SITE #8 - SONIAT CANAL
DSR-051-05-022K - JEFFERSON PARISH



SITE PLAN - SITE # 8
EWP - HURRICANE KATRINA
DSR-051-05-022K-SONIAT CANAL
JEFFERSON PARISH, LOUISIANA

	DATE
DESIGNED JIMMY MOORE	03/07
DRAWN ABBEY GREMILLION	04/07
CHECKED CHERIE LAFLEUR	04/07
APPROVED ED GIERING III	04/07

FILE NAME
LA051-05-022K-32
DRAWING NAME
LA051-05-022K-32
Sheet 32 of 34

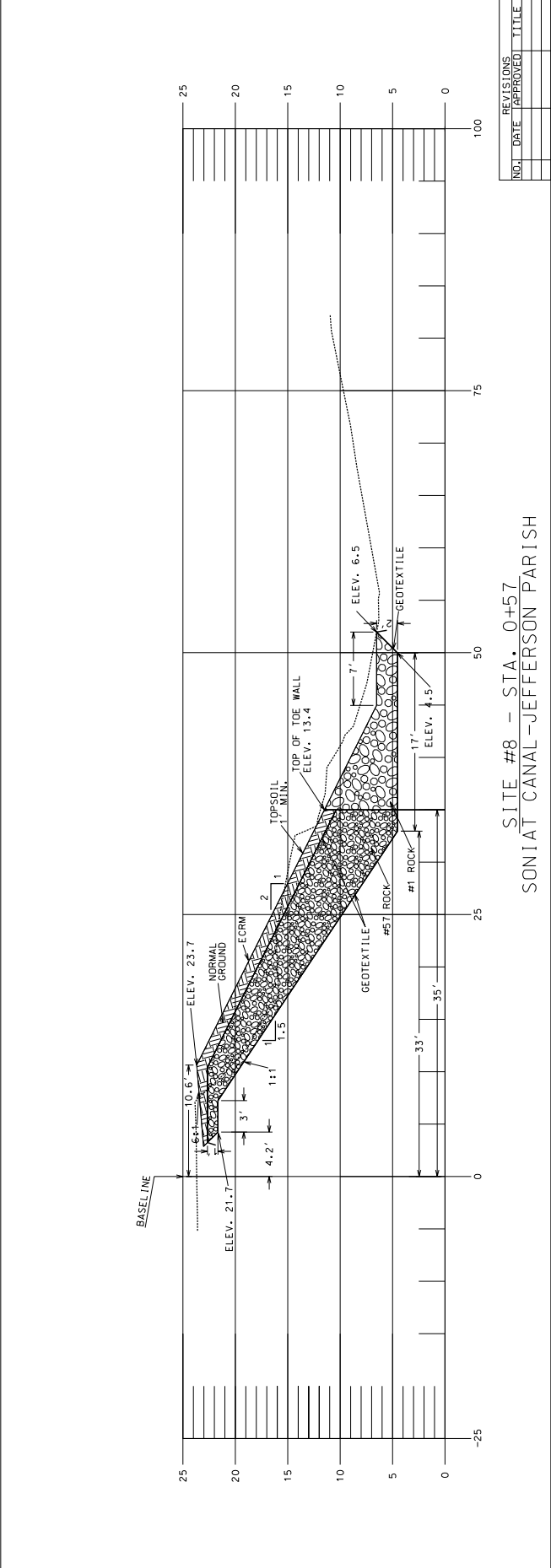
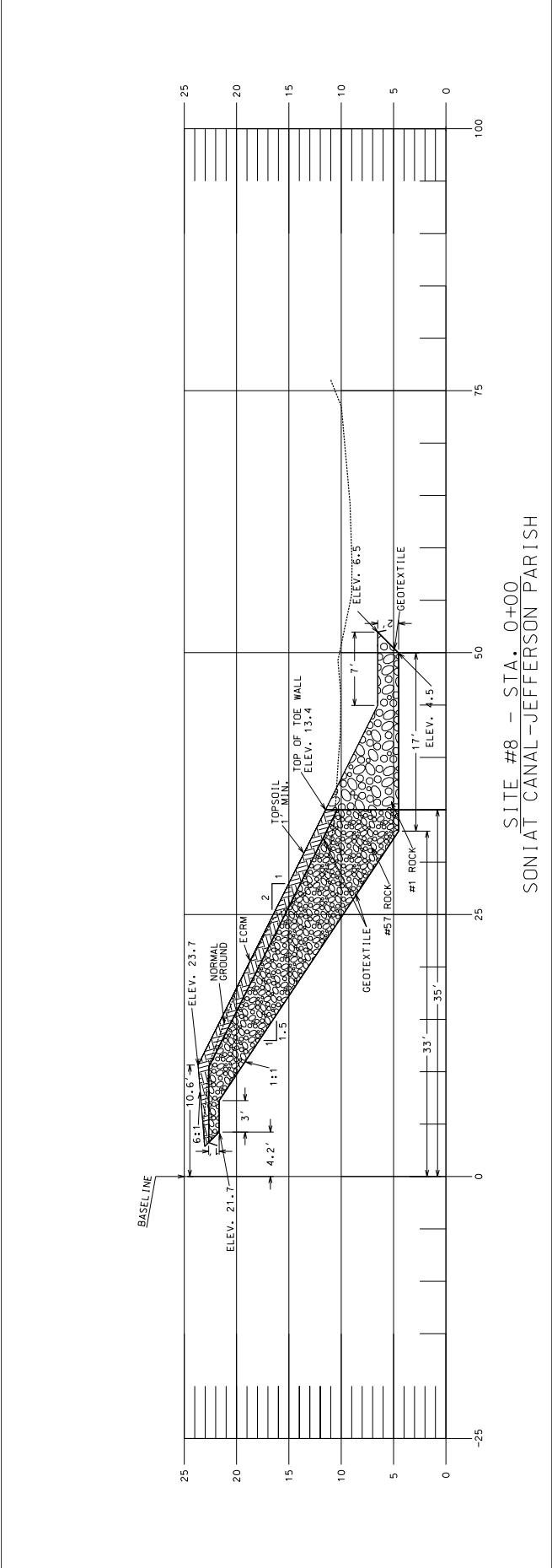
DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE #8 SECTIONS - STA. 0+00 & 0+57
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-33
DRAWING NAME	LA051-05-022K-33

NO.	DATE	APPROVED	TITLE

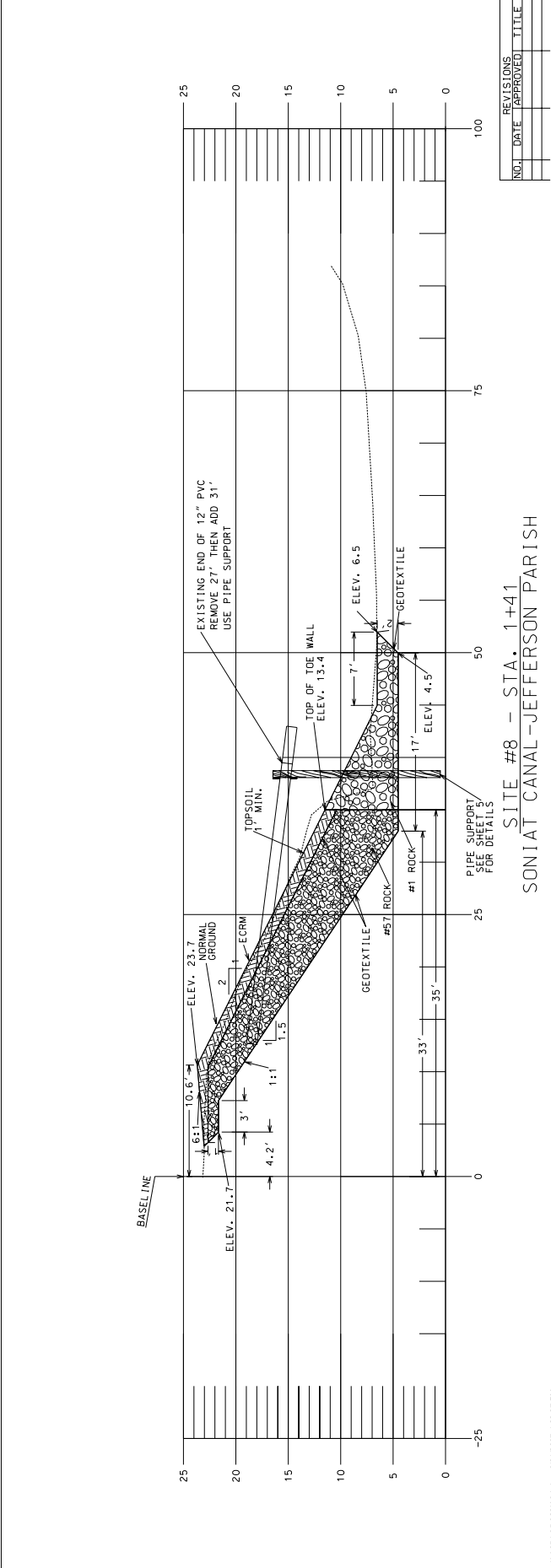
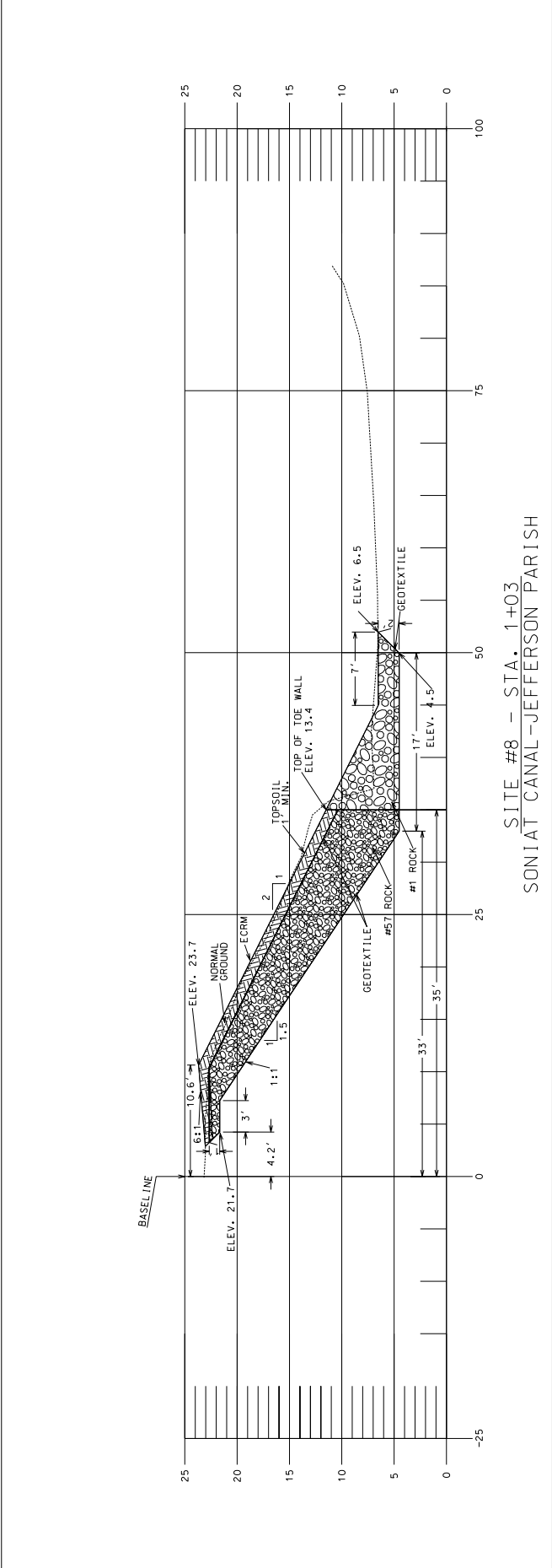


DATE	03/07
DESIGNED	JIMMY MOORE
DRAWN	MATTHEW SPRINGER
CHECKED	CHERIE LAFLEUR
APPROVED	ED GIERING, III

SITE #8 SECTIONS - STA. 1+03 & 1+41
 EWP - HURRICANE KATRINA
 DSR051-05-022K - SONIAT CANAL
 JEFFERSON PARISH, LOUISIANA



FILE NAME	LA051-05-022K-34
DRAWING NAME	LA051-05-022K-34



SITE #8 - STA. 1+41
 SONIAT CANAL-JEFFERSON PARISH

NO.	DATE	APPROVED	TITLE

PART I - THE SCHEDULE**SECTION B - SUPPLIES OR SERVICES AND PRICE/COSTS**

EMERGENCY WATERSHED PROTECTION

JEFFERSON PARISH
JEFFERSON PARISH, LADSR -- 051-05-022K
Soniata Canal
Sites 1-8

ITEM#	WORK	SPEC	QTY	UNIT	UNIT PRICE	AMOUNT
1	Mobilization & Demobilization	8	1	JOB	LUMP SUM	\$
2	Seeding, Fertilization and Mulching	6	1	JOB	LUMP SUM	\$
3	Excavation	21	1	JOB	LUMP SUM	\$
4	Earthfill, Topsoil	23	1	JOB	LUMP SUM	\$
5	Rockfill	25	1	JOB	LUMP SUM	\$
6	Geotextile	95	1	JOB	LUMP SUM	\$
7	Steel Sheetpile Toewall	13	1	JOB	LUMP SUM	\$
8	Corrugated Metal Pipe	51	1	JOB	LUMP SUM	\$
9	Plastic Pipe	45	1	JOB	LUMP SUM	\$

TOTAL BID \$ _____

B.2 Award Restrictions (AUG 1987, SCS, AMB) One Award for the aggregate of all items will be made under this solicitation

NOTE 1: Bidder must complete all bid items.

NOTE 2: This bid schedule must be turned in with the offer.

NOTE 3: Bids must be (A) submitted on the forms furnished by the Government or on copies of those forms, and (B) manually signed. The person signing a bid must initial each erasure or change appearing on any bid form.

NOTE 4: Any deviation from the above three notes may result in a bid being disqualified.

**Attachments For
Jefferson Parish
DSR # 051-05-022K
Soniata Canal**

CONSTRUCTION SPECIFICATIONS

NUMBER	TITLE	PAGES		
2	Clearing & Grubbing	2-1	to	2-3
3	Structure Removal	3-1	to	3-3
5	Pollution Control	5-1	to	5-6
SWPPP	Storm Water Pollution Prevention Plan	SWPPP-1	to	SWPPP-8
SCACR	Small Construction Activity Completion Report	1 of 1		
6	Seeding, Sprigging, and Mulching	6-1	to	6-5
8	Mobilization and Demobilization	8-1	to	8-2
9	Traffic Control	9-1	to	9-4
11	Removal of Water	11-1	to	11-3
13	Pilings	13-1	to	13-6
21	Excavation	21-1	to	21-5
23	Earthfill	23-1	to	23-6
25	Rockfill	25-1	to	25-4
45	Plastic Pipe	45-1	to	45-8
51	Corrugated Metal Pipe	51-1	to	51-5
83	Timber Fabrication & Installation	83-1	to	83-3
95	Geotextile	95-1	to	95-4

MATERIAL SPECIFICATIONS

NUMBER	TITLE	PAGES		
302	Coconut Fiber Erosion Control Revegetation Mat	302-1		
511	Steel Piles	511-1		
512	Wood Piles	512-1		
547	Plastic Pipe	547-1	to	547-2
551	Coated Corrugated Steel Pipe	551-1		
581	Metal	581-1		
582	Galvanizing	582-1		
584	Structural Timber and Lumber	584-1		
585	Wood Preservatives and Treatment	585-1		
592	Geotextile	592-1	to	592-3

DRAWINGS

1 of 34 34 of 34

SPECIAL PROVISIONS

1 of 2 2 of 2

SPECIAL PROVISIONS

1. Pipelines and other utilities are not shown on the plans. An effort will be made by Jefferson Parish to locate and mark their utilities prior to the start of construction. It is the responsibility of the contractor to contact LA ONE CALL and the appropriate Jefferson Parish Drainage Superintendent to assist in the location of unmarked utilities prior to the start of his/her work.

LOUISIANA ONE CALL

1-800-272-3020

The East Bank Superintendent contacts are:

Bill Duplaisir at (504) 736-6642 or

John Sell at (504) 736-6645

The West Bank Superintendent contact person is:

Larry Palmisamo at (504) 349-5159 or

Lonnie Robinson at (504) 437-4938

2. Unless otherwise specified, the contractor shall be responsible for notifying Jefferson Parish and the respective utility owner(s) to address the protection and/or re-location of utilities within the limits of the work for the duration of the contractor's operations at the project site. The COTR shall concur with any needed adjustments to the proposed work prior to the contractor implementing said adjustments.
3. When performing work in the vicinity of utilities and/or other structures the contractor shall take extreme care not to damage said utilities and/or structures. Any damages resulting from improper construction will be the responsibility of the contractor, and repairs of such damages will be made by the contractor at his/her expense. The contractor shall further restore at his/her own expense all injured property caused by any negligent act of omission or commission on his/her part or on the part of his/her agent, including streets, sidewalks, curbing, sodding, pipes, conduits, shrubs, sewers, buildings, fences, bridges, retaining walls, tanks, power lines, levees or any other building or private property to a condition as good as it was when he/she entered upon the right of way.
4. The convenience of the general public and of residents along the work shall be provided for in a reasonably adequate and satisfactory manner. Where existing roads are not available for use as detours, unless otherwise provided, all traffic shall be permitted to pass through the work. In such cases the vehicles of the traveling public shall have precedence over contractor's vehicles to the end that the traveling public's vehicles shall not be unduly delayed for the convenience of the contractor. In order that all unnecessary delays to the traveling public may be avoided, the contractor shall provide and station competent flagmen whose sole duties shall consist of directing and controlling the movement of public traffic either through or around the work. A flagman shall also be stationed wherever equipment, trucks, etc., enter or leave a thoroughfare from the construction area. No road shall be closed by the contractor to the public except as called for in the specification.
5. The contractor shall arrange his work so that no undue or prolonged blocking of business establishments or private resident will occur. Materials and equipment stored on the right of way or project site shall be so placed and the work at all times shall be so conducted as to insure minimum danger and obstruction to the traveling public.
6. When transporting equipment, supplies and material to and from the construction site, the contractor shall take the most direct route when leaving a major thoroughfare.
7. Fire hydrants shall be accessible at all times to the Fire Department. No materials or other obstructions shall be placed closer to a fire hydrant than permitted by ordinances, rules or regulations or within fifteen (15) feet of a fire hydrant, in the absence of such ordinances, rules or regulations.

8. Material Certifications shall be provided to the Contracting Officer for all materials used in this contract prior to installation.
9. When performing the work at site 4, the contractor shall complete all work necessary to restore the channel slope, as called for in this contract, adjacent to the high voltage power tower first, with the exception that placing topsoil, seeding, fertilizing and mulching may be left to be completed with the rest of site 4. In addition, the excavated slope shall not be left exposed more than 6 hours without the placement of the geotextile and rockfill. The contractor shall notify Daniel Calama, phone (504) 670-3709, of Entergy, at least one week prior to working on site 4 to allow Entergy to have an inspector present on site.
10. The following applies to the payment for excavation, rockfill, earthfill, geotextile, rock riprap, and mulching. The Estimated Quantities shown on the drawings are based on quantities derived from preliminary survey data and are calculated to the finished neat lines and grades on the plans. Variations in these quantities may be possible when the work is actually performed. However, modification to the contract will not be made for work performed in excess of these estimated quantities except under the following conditions:
 - i) The variation must exceed 15% more than the estimated quantity or have a minimum contract value for the additional work in excess of \$500.00. (The contract value is to be determined by dividing the lump sum amount in the Bid Schedule by the total applicable estimated units as shown in the contract item tables of quantities.) If the variation exceeds 15% more than the estimated quantity or \$500.00, a price adjustment can be made for the amount exceeding 15% or \$500.00 more than the original estimated quantity.
 - ii) It shall be the contractor's responsibility to submit proof that the quantity in question exceeds the percentage and cost parameters in above Item i. Proof will consist of applicable survey data or other measurements made by a Registered Professional Engineer or Land Surveyor in accordance with recognized professional practice and standards of the surveying profession.
 - iii) The survey data or other measurements as applicable shall be presented to the Natural Resources Conservation Service (NRCS) prior to any work on the contract item for which the quantity is questioned. One working day shall be provided to the NRCS to verify data prior to the beginning of work for this contract item.

A final survey or other measurements as applicable shall be made and presented to the NRCS after the work is completed, which will allow measurement for the quantity in question. If this survey data indicates justification for a contract modification within the parameters of above Item i, it shall be made in accordance with the contract terms and conditions.

All computations for excavation and fill items shall be computed to the neat lines and grades as shown on the drawings.
11. Soil borings and survey data is available for viewing at the Natural Resources Conservation Service, Emergency Operations Center, 2420 Athania Pkwy, Suite 300, Metairie, Louisiana 70001. Contact the Contract Specialist at 504-828-1866 Ext. 4 to schedule an appointment.
12. Unless otherwise concurred by the COTR, when a project for slope repair/stabilization consists of several sites along the same reach of channel, the contractor shall "work his way out of the project". The work shall begin at the site located the greatest travel distance from the project's entry point off a public road and proceed back towards the entry point. For example, if the access to the channel is off a public road at the north end of the project then the work shall begin at the southernmost site and proceed north. All slope repair work will be conducted to minimize the temporary storage of excavated material and backfill materials above the slope being repaired.
13. Unless otherwise concurred by the COTR, when a project consists of several slope repair/stabilization sites along a channel or canal, the contractor shall complete 80% of all the work at a site prior to beginning any work at the next site.

Construction Specification 2—Clearing and Grubbing

1. Scope

The work consists of clearing and grubbing and disposal of trees, snags, logs, brush, stumps, shrubs, and rubbish from the designated areas.

2. Protection of existing vegetation

Trees and other vegetation designated to remain undisturbed shall be protected from damage throughout the duration of the construction period. Any damages resulting from the contractor's operations or neglect shall be repaired by the contractor.

Earthfill, stockpiling of materials, vehicular parking, and excessive foot or vehicular traffic shall not be allowed within the drip line of vegetation designated to remain in place. Vegetation damaged by any of these or similar actions shall be replaced with viable vegetation of the same species, similar condition, and like size unless otherwise approved by the contracting officer.

Any cuts, skins, scrapes, or bruises to the bark of the vegetation shall be carefully trimmed and local nursery accepted procedures used to seal damaged bark.

Any limbs or branches 0.5 inch or larger in diameter that are broken, severed, or otherwise seriously damaged during construction shall be cut off at the base of the damaged limb or branch flush with the adjacent limb or tree trunk. All roots 1-inch or larger in diameter that are cut, broken, or otherwise severed during construction operations shall have the end smoothly cut perpendicular to the root. Roots exposed during excavation or other operations shall be covered with moist earth or backfilled as soon as possible to prevent the roots from drying out.

3. Marking

The limits of the area(s) to be cleared and grubbed will be marked by stakes, flags, tree markings, or other suitable methods. Trees to be left standing and uninjured will be designated by special markings placed on the trunk about 6 feet above the ground surface.

4. Clearing and grubbing

All trees not marked for preservation and all snags, logs, brush, stumps, shrubs, rubbish, and similar materials shall be cleared from within the limits of the designated areas. Unless otherwise specified, all stumps, roots, and root clusters that have a diameter of 1 inch or larger shall be grubbed out to a depth of at least 2 feet below subgrade for concrete structures and 1 foot below the ground surface at embankment sites and other designated areas.

5. Disposal

All materials cleared and grubbed from the designated areas shall be disposed of at locations shown on the drawings or in a manner specified in section 7. The contractor is responsible for complying with all local rules and regulations and the payment of any and all fees that may result from disposal at locations away from the project site.

6. Measurement and payment

Method 1—For items of work for which specific units prices are established in the contract, the cleared and grubbed area is measured to the nearest 0.1 acre. Payment for clearing and grubbing is made for the total area within the designated limits at the contract unit price. Such payment will constitute full compensation for all labor, equipment, tools, and all other items necessary and incidental to the completion of the work.

Method 2—For items of work for which specific unit prices are established in the contract, the length of the cleared and grubbed area is measured to the nearest full station (100 feet) along the line designated on the drawing or identified in the specifications. Payment for clearing and grubbing is made for the total length within the designated limits at the contract unit price. Such payment will constitute full compensation for all labor, equipment, tools, and all other items necessary and incidental to the completion of the work.

Method 3—For items of work for which specific unit prices are established in the contract, each tree, stump, and snag having a diameter of 4 inches or larger and each log having a diameter of 4 inches or larger and a length of 10 feet are measured before removal. The size of each tree and snag is determined by measuring its trunk at breast height above the natural ground surface. The size of each log is determined by measuring the butt and by measuring its length from butt to tip. The size of each stump is measured at the top. Diameter is determined by dividing the measured circumference by 3.14.

Payment for clearing and grubbing of each tree, stump, and snag having a diameter of 4 inches or larger and each log having a diameter of 4 inches or larger and a length of 10 feet or larger is made at the contract unit price for its size designation as determined by the following schedule:

Measured diameter (in)	Size designation (in)
4 to 8	6
8 to 12	10
12 to 24	18
24 to 36	30
36 to 60	48
Over 60	60

The sum of such payments shall constitute full compensation for clearing and grubbing (including the clearing and grubbing of smaller trees, stumps, snags, logs, brush, shrubs, and roots), applicable permits and associated fees, and rubbish removal. Such payment shall constitute full compensation for all labor, equipment, tools, and all other items necessary and incidental to the completion of the work.

Method 4—For items of work for which specific lump sum prices are established in the contract, payment for clearing and grubbing is made at the contract lump sum price. Such payment shall constitute full compensation for all labor, equipment, tools, and all other items necessary and incidental to the completion of the work.

All Methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule will be included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 7.

7. Items of work and construction details

7. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Subsidiary Item, Clearing and Grubbing

- (1) This item shall consist of clearing and grubbing the construction area as necessary to do the repair work as shown on the construction drawings and as called for by the Specifications.
- (2) All cleared materials and debris (trees, stumps, brush, broken concrete, and other foreign materials), above and below the ground surface, shall be removed from the repair site to the extent necessary to perform the work.
- (3) All cleared materials and debris shall be loaded and hauled to the nearest approved public landfill, that meets all State and Federal requirements. The contractor is responsible for payment of any tipping fees.
- (4) No separate payment will be made for this item. Compensation for Subsidiary Item, "Clearing & Grubbing" will be included in the payment for Bid Item 3, Excavation.

Construction Specification 3—Structure Removal

1. Scope

The work shall consist of the removal, salvage, and disposal of structures (including fences) from the designated areas.

2. Marking

Method 1—Each structure or structure part to be removed will be marked with stakes, flags, paint, or other suitable method.

Method 2—The area boundaries from which structures must be removed will be marked using stakes, flags, paint, or other suitable method. Structures to remain undisturbed or to be salvaged will be designated by special markings.

3. Removal

Method 1—All structures designated for removal in the contract shall be removed to the specified extent and depth.

Method 2—Within the areas so marked, all visible and buried structures identified shall be removed to the specified extent and depth.

4. Salvage

Structures or structure parts that are designated to be salvaged shall be carefully removed and neatly placed in the specified or approved storage location. Salvaged structures that are capable of being disassembled shall be dismantled into individual members or sections. Such structures shall be neatly and systematically match marked with paint before disassembly. All connectors and other parts shall be marked to indicate their proper location within the structure and shall be fastened to the appropriate structural member or packed in suitable containers.

Material from fences designated to be salvaged shall be placed outside the work area on the property on which the fence was originally located. Fence wire shall be rolled into uniform rolls of suitable size and neatly piled with other salvaged materials. Posts and rails shall be neatly stacked.

5. Disposal of refuse materials

Refuse materials resulting from structure removal shall be disposed of in a manner and at locations specified in section 7 of this specification or in an acceptable manner and at locations approved by the contracting officer. Disposal by burning shall be in accordance with local rules and regulations.

6. Measurement and payment

Method 1—For items of work for which specific unit prices are established by the contract, payment for the removal of each structure unit, except fences, is made at the contract unit price. Fences removed or removed and salvaged are measured to the nearest linear foot. Payment for fence removal or removal and salvage is made at the contract unit prices for each type and size of fence.

Such payment will constitute full compensation for all labor, equipment, tools, applicable permits and associated fees for burning and disposal of refuse, and all other items necessary and incidental to the completion of the work.

Method 2—For items of work for which specific lump sum prices are established by the contract, payment for structure removal is made at the contract lump sum price.

Such payment will constitute full compensation for all labor, equipment, tools, applicable permits and associated fees for burning and disposal of refuse, and all other items necessary and incidental to the completion of the work.

All Methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed as a contract line item number in the bid schedule, is included in the payment for the item of work to which it is made subsidiary. Such items and items to which they are made subsidiary are identified in section 7 of this specification.

7. Items of work and construction details

7. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Subsidiary Item, Structure Removal

- 1) This item shall consist of removal and disposal of steel sheet piles, timber toewalls, corrugated metal pipe and plastic pipe to the extent shown on the drawings.
- 2) Existing pipes shall be fully supported before the removal of the existing pipe supports.
- 3) Marking shall be by Method 1. Each structure to be removed will be marked by means of stakes, flags, painted markers or other suitable methods by the COTR.
- 4) Removal shall be by Method 2. The extent of depth of removal shall be that required to complete the repair as shown on the drawings.
- 5) While no existing timber toe walls were detected during the inspection process if any are encountered during construction the extent of depth of removal shall be that required to complete the repair as shown on the drawings. The depth of removal of the timber toe wall shall be as a minimum of 1' below the excavation limits at the site.
- 6) Salvaging of materials will not be required.
- 7) Refuse material resulting from structure removal shall be loaded and hauled to the nearest approved public landfill, that meets all State and Federal requirements
- 8) No separate payment will be made for this item. Compensation for Subsidiary Item, "Structure Removal" will be included in the payment for Bid Item 7, Piling, Toe Wall Slope Repair.

Construction Specification 5—Pollution Control

1. Scope

The work consists of installing measures or performing work to control erosion and minimize the production of sediment and other pollutants to water and air from construction activities.

2. Material

All material furnished shall meet the requirements of the material specifications listed in section 8 of this specification.

3. Erosion and sediment control measures and works

The measures and works shall include, but are not limited to, the following:

Staging of earthwork activities—The excavation and moving of soil materials shall be scheduled to minimize the size of areas disturbed and unprotected from erosion for the shortest reasonable time.

Seeding—Seeding to protect disturbed areas shall occur as soon as reasonably possible following completion of that earthwork activity.

Mulching—Mulching to provide temporary protection of the soil surface from erosion.

Diversions—Diversions to divert water from work areas and to collect water from work areas for treatment and safe disposition. They are temporary and shall be removed and the area restored to its near original condition when the diversions are no longer required or when permanent measures are installed.

Stream crossings—Culverts or bridges where equipment must cross streams. They are temporary and shall be removed and the area restored to its near original condition when the crossings are no longer required or when permanent measures are installed.

Sediment basins—Sediment basins collect, settle, and eliminate sediment from eroding areas from impacting properties and streams below the construction site(s). These basins are temporary and shall be removed and the area restored to its original condition when they are no longer required or when permanent measures are installed.

Sediment filters—Straw bale filters or geotextile sediment fences trap sediment from areas of limited runoff. Sediment filters shall be properly anchored to prevent erosion under or around them. These filters are temporary and shall be removed and the area restored to its original condition when they are no longer required or when permanent measures are installed.

Waterways—Waterways for the safe disposal of runoff from fields, diversions, and other structures or measures. These works are temporary and shall be removed and the area restored to its original condition when they are no longer required or when permanent measures are installed.

Other—Additional protection measures as specified in section 8 of this specification or required

by Federal, State, or local government.

4. Chemical pollution

The contractor shall provide watertight tanks or barrels or construct a sump sealed with plastic sheets to dispose of chemical pollutants, such as drained lubricating or transmission fluids, grease, soaps, concrete mixer washwater, or asphalt, produced as a by-product of the construction activities. At the completion of the construction work, sumps shall be removed and the area restored to its original condition as specified in section 8 of this specification. Sump removal shall be conducted without causing pollution.

Sanitary facilities, such as chemical toilets, or septic tanks shall not be located next to live streams, wells, or springs. They shall be located at a distance sufficient to prevent contamination of any water source. At the completion of construction activities, facilities shall be disposed of without causing pollution as specified in section 8 of this specification.

5. Air pollution

The burning of brush or slash and the disposal of other materials shall adhere to state and local regulations.

Fire prevention measures shall be taken to prevent the start or spreading of wildfires that may result from project activities. Firebreaks or guards shall be constructed and maintained at locations shown on the drawings.

All public access or haul roads used by the contractor during construction of the project shall be sprinkled or otherwise treated to fully suppress dust. All dust control methods shall ensure safe construction operations at all times. If chemical dust suppressants are applied, the material shall be a commercially available product specifically designed for dust suppression and the application shall follow manufacturer's requirements and recommendations. A copy of the product data sheet and manufacturer's recommended application procedures shall be provided to the engineer 5 working days before the first application.

6. Maintenance, removal, and restoration

All pollution control measures and temporary works shall be adequately maintained in a functional condition for the duration of the construction period. All temporary measures shall be removed and the site restored to near original condition.

7. Measurement and payment

Method 1—For items of work for which specific unit prices are established in the contract, each item is measured to the nearest unit applicable. Payment for each item is made at the contract unit price for that item. For water or chemical suppressant items used for dust control for which items of work are established in section 8 of this specification, measurement for payment will not include water or chemical suppressants that are used inappropriately or excessive to need. Such payment will constitute full compensation for the completion of the work.

Method 2—For items of work for which lump sum prices are established in the contract, payment is made as the work proceeds and supported by invoices presented by the contractor that reflect actual costs. If the total of all progress payments is less than the lump sum contract price for this item, the balance remaining for this item will be included in the final contract payment. Payment of the lump sum contract price will constitute full compensation for completion of the

work.

Method 3—For items of work for which lump sum prices are established in the contract, payment will be prorated and provided in equal amounts on each monthly progress payment estimate. The number of months used for prorating shall be the number estimated to complete the work as outlined in the contractor's approved construction schedule. The final month's prorate amount will be provided with the final contract payment. Payment as described will constitute full compensation for completion of the work.

All Methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items, and the items to which they are made subsidiary, are identified in section 8 of this specification.

8. Items of work and construction details

(See next page.)

8. ITEMS OF WORK AND CONSTRUCTION DETAILS

It has been determined that this project comes under the LDEQ Louisiana Pollution Discharge Elimination System (LPDES), and that a Storm Water Pollution Prevention Plan is required for this project since the disturbed area will be greater than 1 acres in size, but less than 5 acres in size. Please note that the rules for LPDES process relative to construction sites are contained in the LAC Title 33:IX.2341. Rules for storm water discharges associated with construction sites covered by general permits are found in LAC Title 33:IX.2345.

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Subsidiary Item, Pollution Control.

- (1) This item shall consist of all work necessary to control erosion and sediment pollution, chemical pollution, water pollution, and air pollution during the period of this contract. The contractor shall perform the work in a manner that will reduce erosion, minimize sediments and other pollutants to the water and streams, and create a minimum of air pollution. As a part of this requirement, a Storm Water Pollution Prevent Plan shall (SWPPP) be developed by the Contractor in compliance with the following sections.

(2) SWPPP Requirements

A. General SWPPP Requirements

The contents of the SWPPP shall address all of the applicable items identified in Part III of Permit No. LAR200000. Attached is a draft copy of an SWPPP the Contractor may used to develop the site specific SWPPP for implementation on this contract.

B. Scope

The purpose of the SWPPP is to control soil erosion and the resulting sediment from leaving the project work area and prevent pollution of any water body caused by the runoff from the area of construction activities under this contract, under the terms of Permit No LAR200000. The Contractor shall develop a site specific SWPPP that corresponds with the proposed construction activities by type and time of occurrence, and implement the SWPPP in a manner which will meet the requirements Permit No. LAR200000. The Contractor shall also assure that all subcontractors have reviewed the plan and that they comply with its provisions.

C. Definitions

Operator:- Any party associated with the construction project that meets either of the following two criteria: (1) the party has operational control over project specification (including the ability to make modifications in the specifications), or (2) the party has day-to-day control of those activities at a project site which are necessary to ensure compliance with the storm water pollution plan or other permit conditions (e.g., they are authorized to direct workers at the site to carry out activities identified in the storm water pollution prevention plan or comply with other permit conditions.)

Project Completion Report – A document that is completed and submitted to the LDEQ certifying that activities were completed in accordance with the requirements of the Clean Water Act and the Louisianan Environmental Quality Act, and specifically LPDES Small Construction General Permit. LAR200000, under which discharges

related to the construction were authorized... The Project Completion Report should be filed when the permittee is no longer the Construction Operator of the contract, or when termination of stormwater discharge has been accomplished.

D. Record Retention Requirements

Records of the Project Completion Report as well as any data used to complete it, the SWPPP, and any reports required by Permit No. LAR200000 shall be retained by the permittee for at least three years from the date that the site is finally stabilized. Certification of the SWPPP by the Contractor or any sub-contractor is required in accordance with Permit No. LAR200000.

E. Plan Accessibility

The Contractor shall post a notice near the main entrance of the construction site with the following information:

- The LDPEs permit number (LAR200000) and effective date of this permit
- The name and telephone number of a local contact person
- A brief description of the project
- The location of the SWPPP

A copy of the SWPPP required by the permit, including a copy of the permit language shall be retained at the construction site from the date of construction initiation to the date of stabilization. The permittee with day-to-day operational control over the SWPPP implementation shall have a copy of the plan available at a central location on-site for the use of operators and those identified as having responsibilities under the plan.

F. LDEQ Correspondence

Any written correspondence concerning the NOI, Project Completion Report, SWPPP, or discharges covered under Permit No. LAR200000, shall be identified by permit number, if one has been assigned, and a copy provided to the Contracting Officer.

LDEQ mailing address is as follows:

Louisiana Department of Environmental Quality
Office of Environmental Services
P.O. Box 82135
Baton Rouge, LA 70884-2135
Attn: Permits Division

G. Control Measures

Control measures that will be implemented shall be in compliance with Permit No. LAR200000, and identified in the SWPPP. The control measures shall include erosion control measures for both short and long term erosion control measures (BMP's) both vegetative and structural.

H. Maintenance and Inspection

The Contractor shall maintain the temporary and permanent vegetation, erosion and sediment control measures, and other protective measures in good and effective operating condition. The Contractor shall inspect disturbed areas of the construction site, areas used for storage of materials, stabilization practices, structural practices, and other controls at least once every fourteen (14) calendar days, before anticipated storm

events expected to cause a significant amount of runoff, and within 24 hours of the end of any storm that produces 0.5 inches or more of precipitation.

A report of each inspection shall be made and included with the daily QC report. Any items identified in the inspection requiring repairs or restoration shall be immediately corrected and actions taken reported in the daily QC report.

I. Project Completion Report

Upon stabilization and elimination of all storm water discharges authorized by Permit No. LAR200000, a Project Completion Report shall be certified and submitted by the Contractor to the Permits Division of LDEQ. Certified mail is recommended as proof of the Project Completion Report submittal. A copy of the Contractor's Project Completion Report submittal shall be provided to the Contracting Officer at the time of submittal, prior to final acceptance of the work.

- (3) Silt fences shall be installed in the locations shown on the site specific SWPPP. Silt fences shall be installed in accordance with the drawings and maintained until final stabilization is accomplished.
- (4) Hay bales shall be installed in the locations shown on the site specific SWPPP. Hay bales shall be installed in accordance with the drawings and maintained until final stabilization is accomplished
- (5) All paints and hazardous materials shall be kept in the original containers and tightly sealed with the manufacturer's label attached. These must be properly stored when not in use. They shall also be stored in a neat orderly manner in their original containers. Disposal of surplus materials shall be in accordance with the manufacturer's or State and Local regulations and recommended methods. Containers shall be empty before disposal.
- (6) Petroleum products such as fuels and lubricants will be stored in tightly sealed containers that are clearly labeled. The storage and dispensing of all petroleum products will be in accordance with part 1926.152 of the OSHA Construction Industry Safety and Health Standards. All spills will be cleaned up on the same workday of the spill occurrence or whenever discovered.
- (7) Soils contaminated with petroleum products will be removed from the site and disposed of in accordance with State and Local regulations.
- (8) All onsite vehicles and equipment shall be monitored for leaks and receive regular preventive maintenance to reduce the chance for leakage. Leaks shall be repaired as soon as they are identified.
- (9) Sumps used to control chemical pollution shall be sealed with plastic sheets having a minimum thickness of 20 mils.
- (10) The contractor shall anchor all temporary materials used for pollution control in such a manner to prevent its being transported off the worksite by storm runoff water. Damage caused by clogging of downstream bridges and/or culverts by such temporary materials being transported downstream by storm water shall be the responsibility of the contractor.
- (11) No separate payment will be made for this item. Compensation for Subsidiary Item, Pollution Control will be included in the payment for Bid Item 2, Seeding, Fertilization and Mulching.

DRAFT

**STORM WATER POLLUTION
PREVENTION PLAN**

**DSR 051-05-022K
SONIAT CANAL
JEFFERSON PARISH
CHANNEL SLOPE REPAIR**

DATE

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1.1

1.0 SITE OWNER

Owner's Name and Address: (Permanent)

Jefferson Parish
PO Box 9
Jefferson, Louisiana 70054

Owner's Name and Address: (During Construction)

USDA Natural Resources Conservation Service
3737 Government St.
Alexandria, LA 71302

2.0 SWPPP COORDINATOR AND DUTIES

The construction site SWPPP coordinator for the Soniat Canal Channel Slope Repair Project is (**Contractor inserts appropriate persons name, title, contact no., etc.**). Mr. _____ duties include the following:

- Implement the SWPPP
- Oversee installation of control measures
- Conduct inspections of control measures
- Identify deficiencies in the SWPPP or control measures and take corrective action

2.1 SITE LOCATION

The project site is located in Jefferson Parish south of West Metairie Avenue and 300 feet west of David Drive. The site is an existing earthen drainage channel and adjacent top bank in Section 42, Township 12S, Range 10E. The drainage channel is a component of the Jefferson Parish drainage system. The approximate coordinates of the site range:

Latitude 29.96962253° North, and Longitude 90.19568592° West
Latitude 29.98398626° North, and Longitude 90.21427439° West

The location map and site plan within the construction plans of the Soniat Canal Channel Slope Repair contract will act as the site map.

2.2 CONSTRUCTION TYPE

The construction will consist of excavating soil on the failed slopes of the existing channel slopes. The channel slopes will then be rebuilt within the confines of the existing channel bank by constructing a sheet pile toewall and filling with rock and earthfill. All bare areas caused by the excavation and filling operations will be seeded to permanent vegetation and slopes steeper than 6:1 will be mulched.

The total area estimated to be impacted by the construction activities is 1.4 acre.

2.3 EXISTING CONDITIONS

The area in which construction will take place is an existing earthen drainage channel and is a component of the Jefferson Parish drainage system. The site is currently experiencing settlement and movement of soil from the channel slope into the cross section of the channel resulting in decreased channel capacity.

The receiving water for any runoff from this project site of Soniat Canal is pumped into Lake Pontchartrain.

The NRCS Runoff Curve Number for the construction site prior to construction is estimated at 89 for grass with less than 50% cover on a hydrologic group D soil. After construction the RCN is estimated to be 80 for grass with more than 75% cover on hydrologic group D soils.

2.4 CONSTRUCTION SEQUENCE

The proposed construction activities will be in the following sequence with the estimated quantities for each item:

ITEM	QUANTITY	UNIT
Excavation & Place on Top Bank	9339	CY
Toewall	1426	LF
Geotextile	12970	SY
Rockfill #1	4413	CY
Rockfill #57	4757	CY
Earthfill	1589	CY
Seeding	1.4	AC
Mulch	4830	SY

3.0 ENDANGERED OR THREATENED SPECIES

The proposed construction activities are in the vicinity of 10 known and 1 possible endangered or threatened species.

By implementation of this construction, the long term impacts to the fisheries species will be reduced because the current blockage of the channel cross section and reduced fisheries habitat will be eliminated. With the use of proper listed control measures the amount of sediment leaving the site during construction will be no greater than that presently leaving the site from the slope failure.

4.0 POTENTIAL CONTAMINANTS

The following list of materials or substances are expected to be present during construction which could impact water or air quality if improperly used.

- Petroleum Based Products
- Earthfill (sediment)
- Fertilizers

The contractor will be responsible for spill prevention and cleanup. The contractor will submit an emergency response plan to the Contracting Officer prior to the start of work on this contract. The contractor's emergency response plan will be clearly posted and site personnel will be made aware of the procedures and the location of the information and cleanup supplies.

The following are the Material Management Practices that will be used to reduce the risk of spills or other accidental exposure of materials and substances to storm runoff water:

An effort will be made to store only enough products required to do the job. All materials stored on site will be stored in a neat, orderly manner in their appropriate containers and, if possible under a roof or other enclosure. Products will be kept in their original containers with the manufacturers' label. Manufacturers' recommendations for proper use and disposal will be followed. Containers of products will be empty before disposal.

The following additional Practices will be used to reduce the risks associated with hazardous materials:

Hazardous products will be kept in original containers unless containers cannot be resealed. Original labels and materials safety data will be retained. Surplus products and containers will be properly disposed of in accordance with manufacturers' or State and local regulations and recommended methods. Containers will be empty before disposal.

Petroleum Products:

All on site vehicles will be monitored for leaks and receive regular preventive maintenance to reduce the chance of leakage. Petroleum products will be stored in tightly sealed containers, which are clearly labeled. Any asphalt substances used onsite will be applied according to manufacturer's recommendations.

All spills of petroleum products will be cleaned up immediately. All contaminated soils will be removed from the site and disposed of in accordance with State and local regulations.

Fertilizers:

Fertilizers will be applied in the amount and at the rate recommended in the project specifications. These rates shall not exceed the manufacturers' recommendation. Storage will be in a covered shed. The contents of any partially used bags of fertilizer will be transferred to an acceptable sealable plastic container to avoid spills.

Spill Control Practices - The following additional practices will be followed for spill prevention and cleanup:

Manufacturers' recommended methods for spill cleanup will be clearly posted and site personnel will be made aware of the procedures and the location of the information and cleanup supplies. Materials and equipment necessary for spill cleanup will be available onsite.

All spills of hazardous materials will be cleaned up immediately after discovery. Spills of toxic or hazardous materials will be reported to the appropriate State and local government agency. The contractor will be responsible for spill prevention and cleanup.

5.0 CONTROLS TO REDUCE POLLUTANTS

The contractor will be required to prepare a written plan for pollution control at the project site. The plan will outline construction sequences and construction activities so

that the least area possible is disturbed by the various construction activities in the course of the construction of the project. It will contain management provisions for storm water POLLUTION control.

It is the responsibility of the Contractor to develop a **Site Specific Storm Water Pollution Prevention Plan** around his proposed construction operations. The following Erosion and Sediment Control plan is offered for consideration by the Contractor in the development of his plan for the construction of the embankment and installation of the structures. The contractor is reminded that **this is a draft plan only** and is not intended to dictate a construction sequence or any construction activities.

Temporary Erosion and Sediment Controls:

- Sediment fences and or hay bail barriers shall be installed along the perimeter of the areas to be cleared and graded before any clearing or grading takes place.
- Sediment fences and or hay bail barriers shall be installed along the perimeter of the temporary spoil pile areas. The sediment containment measures will be installed prior to the placement of spoil within the area.
- Any areas where anticipated work will not occur for 14 days will be temporarily seeded in accordance with Specification 6 of the construction contract.
- Upon completion of shaping and grading activities, all bare areas will be fertilized, seeded and mulched in accordance with Specification 6 of the construction contract.

Waste Disposal

- All chemical, hazardous and sanitary waste materials will be disposed of in an approved offsite disposal area. Chemical waste shall be temporarily stored in leak proof containers until disposed of in an approved area.
- Accidental chemical spills will be properly cleaned up on the same day of occurrence. Daily inspection will be made to determine needed cleanup.
- Sanitary waste will be collected from portable units a minimum of two times per week to avoid overfilling.

6.0 CERTIFICATION OF COMPLIANCE WITH REGULATIONS

All local and state regulations will be adhered to concerning the burning of organic materials or disposal of organic, chemical, and sanitary waste. This project has been authorized by the Department of the Army in accordance with Section 404 of the Clean Water Act (CWA). The State of Louisiana, Department of Environmental Quality, has issued a Water Quality Certification permit. There are no other applicable State or Federal requirements for sediment and erosion site plans or storm water management site plans.

7.0 **MAINTENANCE AND INSPECTION PROCEDURES**

The contractor will be responsible for intermittent review and inspection of the operation and maintenance of all pollution control measures throughout the life of the contract. Visual inspections of all cleared and graded areas of the construction site will be performed daily. Also inspection of the conditions and the need for repair shall be made after each storm rainfall exceeding 0.5 inch. Daily inspections of the need for clean up of chemical spills and sanitary facilities are specified.

The inspections will verify that the procedures used to prevent storm water contamination from construction materials are effective. Any items requiring maintenance will be immediately addressed.

8.0 CERTIFICATION

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system design to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manages the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Signature: _____

Date: _____

Name: _____

Title: _____

Firm: _____

Address: _____

Phone: _____

9.0 SUBCONTRACTOR CERTIFICATION

I certify under penalty of law that I understand the terms and conditions of the general Louisiana Pollution Discharge Elimination Systems (LPDES) permit that authorizes the storm water discharges associated with industrial activity from the construction site identified as part of this certification.

Signature: _____

Date: _____

Name: _____

Title: _____

Firm: _____

Address: _____

Phone: _____

Signature: _____

Date: _____

Name: _____

Title: _____

Firm: _____

Address: _____

Phone: _____



STATE OF LOUISIANA
DEPARTMENT OF ENVIRONMENTAL QUALITY
Office of Environmental Services, Permits Division
Post Office Box 4313
Baton Rouge, Louisiana 70821-4313
PHONE#: (225) 219-3181
FAX#: (225) 219-3309



**SMALL CONSTRUCTION ACTIVITY
COMPLETION REPORT LAR200000**

To be submitted by January 28 in the year FOLLOWING COMPLETION of covered activities.

Section I – Operator Information

Name: _____

Mailing Address: _____

City: _____ State: _____ Zip: _____

Phone Number: _____ DEQ AI# (if known): _____

Section II – Facility/Site Information

Name of Project: _____

Location of Project: _____

City: _____ State: _____ Zip: _____

Parish: _____ Name of Receiving Water: _____

Total Area of Land Disturbance (in acres) _____

Construction Start Date: _____

Construction Completion/Site Stabilization Date: _____

List existing or prior water discharge permits for the location: _____

Section III - Certification

I certify under penalty of law that project activities were completed in accordance with the requirements of the Clean Water Act and the Louisiana Environmental Quality Act, and specifically in accordance with the LPDES Small Construction General Permit, LAR200000, under which the storm water discharges related to the construction were authorized. I understand that submittal of this Report does not release an Operator from liability for any violation of the permit or the Act. I further certify that the information submitted is, to the best of my knowledge and belief, true, accurate, and complete, and that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Printed Name: _____

Signature: _____

Date: _____

Construction Specification 6—Seeding, Sprigging, and Mulching

1. Scope

The work consists of preparing the area for treatment; furnishing and placing seed, sprigs, mulch, fertilizer, inoculant, lime, and other soil amendments; and anchoring mulch in designated areas as specified.

2. Material

Seed—All seed shall conform to the current rules and regulations of the state where it is being used and shall be from the latest crop available. It shall meet or exceed the standard for purity and germination listed in section 7.

Seed shall be labeled in accordance with the state laws and the U.S. Department of Agriculture rules and regulations under the Federal Seed Act in effect on the date of invitations for bids. Bag tag figures are evidence of purity and germination. No seed will be accepted with a test date of more than 9 months before the delivery date to the site.

Seed that has become wet, moldy, or otherwise damaged in transit or storage will not be accepted. The percent of noxious weed seed allowable shall be as defined in the current State laws relating to agricultural seeds. Each type of seed shall be delivered in separate sealed containers and fully tagged unless exception is granted in writing by the contracting officer.

Fertilizer—Unless otherwise specified, the fertilizer shall be a commercial grade fertilizer. It shall meet the standard for grade and quality specified by State law. Where fertilizer is furnished from bulk storage, the contractor shall furnish a supplier's certification of analysis and weight. When required by the contract, a representative sample of the fertilizer shall be furnished to the contracting officer for chemical analysis.

Inoculants—The inoculant for treating legume seeds shall be a pure culture of nitrogen-fixing bacteria prepared specifically for the species and shall not be used later than the date indicated on the container or as otherwise specified. A mixing medium, as recommended by the manufacturer, shall be used to bond the inoculant to the seed. Two times the amount of the inoculant recommended by the manufacturer shall be used except four times the amount shall be used when seed is applied using a hydraulic seeder. Seed shall be sown within 24 hours of treatment and shall not remain in the hydraulic seeder longer than 4 hours.

Lime and other soil amendments—Lime shall consist of standard ground agriculture limestone, or approved equivalent. Standard ground agriculture limestone is defined as ground limestone meeting current requirements of the State Department of Agriculture. Other soil amendments shall meet quality criteria and application requirements specified in section 7.

Mulch tackifiers—Asphalt emulsion tackifiers shall conform to the requirements of ASTM D 977, Specification for Emulsified Asphalt. The emulsified asphalt may be rapid setting, medium setting, or slow setting. Nonasphaltic tackifiers required because of environmental considerations shall be as specified in section 7.

Straw mulch material—Straw mulch shall consist of wheat, barley, oat or rye straw, hay, grass cut from native grasses, or other plants as specified in section 7. The mulch material shall be air-

dry, reasonably light in color, and shall not be musty, moldy, caked, or otherwise of low quality. The use of mulch that contains noxious weeds is not permitted. The contractor shall provide a method satisfactory to the contracting officer for determining weight of mulch furnished.

Other mulch materials—Mulching materials, such as wood cellulose fiber mulch, mulch tackifiers, synthetic fiber mulch, netting, and mesh, are other mulching materials that may be required for specialized locations and conditions. These materials, when specified, must be accompanied by the manufacturer's recommendations for methods of application.

3. Seeding mixtures, sod, sprigs, and dates of planting

The application rate per acre for seed mixtures, sprigs, or sod and date of seeding or planting shall be as shown on the plans or as specified in section 7.

4. Seedbed preparation and treatment

Areas to be treated shall be dressed to a smooth, firm surface. On sites where equipment can operate on slopes safely, the seedbed shall be adequately loosened (4 to 6 inches deep) and smoothed. Depending on soil and moisture conditions, disking or cultipacking, or both, may be necessary to properly prepare a seedbed. Where equipment cannot operate safely, the seedbed shall be prepared by hand methods by scarifying to provide a roughened soil surface so that broadcast seed will remain in place.

If seeding is to be accomplished immediately following construction operations, seedbed preparation may not be required except on a compacted, polished, or freshly cut soil surface.

Rocks larger than 6 inches in diameter, trash, weeds, and other debris that will interfere with seeding or maintenance operations shall be removed or disposed of as specified in section 7.

Seedbed preparation shall be discontinued when soil moisture conditions are not suitable for the preparation of a satisfactory seedbed as determined by the contracting officer's technical representative (COTR).

5. Seeding, sprigging, fertilizing, mulching, and stabilizing

All seeding or sprigging operations shall be performed in such a manner that the seed or sprigs are applied in the specified quantities uniformly in the designated areas. The method and rate of seed application shall be as specified in section 7. Unless otherwise specified, seeding or sprigging shall be accomplished within 2 days after final grading is completed and approved.

Fertilizer, lime, and other soil amendments shall be applied as specified in section 7. When specified, the fertilizer and soil amendments shall be thoroughly incorporated into the soil immediately following surface application.

The rate, amount, and kind of mulching or mesh shall be as specified in section 7. Mulches shall be applied uniformly to the designated areas. They shall be applied to areas seeded not later than 2 working days after seeding has been performed. Straw mulch material shall be stabilized within 24 hours of application using a mulch crimper or equivalent anchoring tool or by a suitable tackifier. When the mulch crimper or equivalent anchoring tool is used, it shall have straight blades and be the type manufactured expressly for and capable of firmly punching the mulch into the soil. Where the equipment can be safely operated, it shall be operated on the contour. Hand methods shall be used where equipment cannot safely operate to perform the work required.

The tackifier shall be applied uniformly over the mulch material at the specified rate, or it shall be injected into the mulch material as it is being applied. Mesh or netting stabilizing materials shall be applied smoothly, but loosely on the designated areas. The edges of these materials shall be buried or securely anchored using spikes or staples as specified in section 7.

The contractor shall maintain the mesh or netting areas until all work under the contract has been completed and accepted. Maintenance shall consist of the repair of areas damaged by water erosion, wind, fire, or other causes. Such areas shall be repaired to reestablish the intended condition and to the design lines and grades required by the contract. The areas shall be refertilized, reseeded, and remulched before the new application of the mesh or netting.

6. Measurement and payment

Method 1—For items of work for which specific unit prices are established in the contract, each area treated is measured as specified in section 7 and the area calculated to the nearest 0.1 acre. Payment for treatment is made at the contract unit price for the designated treatment, which will constitute full compensation for completion of the work.

When specified as an item of work, mesh or netting is measured to the nearest square yard of surface area covered and accepted. Payment is made at the contract unit price and will constitute full compensation for completion of the work.

Method 2—For items of work for which specific lump sum prices are established in the contract, the quantity of work will not be measured for payment. Payment for this item is made at the contract lump sum price for the item and will constitute full compensation for the completion of the work.

Method 3—For items of work for which lump sum prices are established in the contract, payment is made as the work proceeds. Progress payments will be determined as specified in section 7. Payment of the lump sum contract price will constitute full compensation for completion of the work.

All Methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the item(s) to which they are made subsidiary are identified in section 7.

7. Items of work and construction details

(See next sheet)

7. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefor are:

Bid Item 2, Seeding, Fertilization, and Mulching

This item will consist of furnishing and applying seeds, fertilizer, and mulch according to the following specifications:

a. Seeding

- (1) Seeding will be done on all bare areas such as channel slopes, berms, spoil placement areas, access routes, and any other disturbed areas. Seed will not be applied to areas with perennial ponded water.
- (2) No seedbed preparation will be required if the construction equipment has produced a scarified surface and the seeding is done the day the areas to be seeded are worked. If the construction equipment has produced a slick surface, or seeding is not done the day the areas are worked, a seedbed will be prepared by scarifying the soil surface with a spike-tooth harrow or similar implement to a depth of one (1) inch. When more than one species of seed is required, each species shall be seeded separately. Seeds will be applied at the following rates:

Seeding Period	Species	Minimum % pure Live Seed	Pure Live Seed (lb./ac)
Mar 1 - Aug 31	Common Bermuda-grass (hulled)	83	45
Sep 1 - Dec 31	Tall Fescue	80	40
Jan 1 - Feb 28	Common Bermuda-grass(unhulled) and	80	20
	Common Bermuda-grass (hulled) and	83	20
	Rye Grass	82	25

b. Fertilization

- (1) Fertilizer will be applied to all areas to be seeded.
- (2) Fertilizer will be a 1-1-1 ratio of N, P₂O₅, and K₂O, and will contain at least 13 lbs. of each per 100 lbs. of material.

Fertilizer rate (13-13-13 basis)	(14 lbs. per 1000 sq ft) or (600 lbs. per acre)
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- (3) One application of fertilizer will be applied at the time of planting as directed by the COTR.

c. Mulching - Erosion Control Revegetation Mat

- (1) This item consists of furnishing and applying an erosion control revegetation mat (ECRM) on all slopes steeper than 6:1 seeded to permanent vegetation.
 - (2) The ECRM shall conform to the requirements of Material Specification 302, Coconut Fiber Erosion Control Revegetation Mat.
 - (3) The ECRM must provide fiber reinforcement to minimize soil erosion and break up rainfall impact. The material must be open to allow water, air, and sunlight to penetrate and retain sufficient moisture to provide a microclimate for seed germination and plant growth.
 - (4) The ECRM must be stable enough to survive installation and persist in the environment long enough for seed germination and initial establishment of grasses.
 - (5) Final graded slope shall be free of all rocks, clods, vegetation or other obstructions so that the blanket will have direct contact with soil surface.
 - (6) Do not drive tracked or heavy equipment over the ECRM.
 - (7) The ECRM will be anchored as per manufacturer's recommendations with 8 Ga., 8 in. x 2 in. x 8 in. wire staples.
 - (8) ECRM will be placed on areas that have been seeded and fertilized within the same workday as the seeding and fertilization.
 - (9) ECRM will be installed up and down the slope and as per manufacturer's recommendations.
- d. Measurement and payment shall be by Method 2. Such payment shall be considered full compensation for all material, labor, equipment, tools and any other items necessary and incidental to the completion of the work. Such payment will constitute full compensation for "Subsidiary Item, Pollution Control".

Construction Specification 8—Mobilization and Demobilization

1. Scope

The work consists of the mobilization and demobilization of the contractor's forces and equipment necessary for performing the work required under the contract. It does not include mobilization and demobilization for specific items of work for which payment is provided elsewhere in the contract. Mobilization will not be considered as work in fulfilling the contract requirements for commencement of work.

2. Equipment and material

Mobilization shall include all activities and associated costs for transportation of contractor's personnel, equipment, and operating supplies to the site; establishment of offices, buildings, and other necessary general facilities for the contractor's operations at the site; premiums paid for performance and payment bonds including coinsurance and reinsurance agreements as applicable; and other items specified in section 4 of this specification.

Demobilization shall include all activities and costs for transportation of personnel, equipment, and supplies not required or included in the contract from the site; including the disassembly, removal, and site cleanup of offices, buildings, and other facilities assembled on the site specifically for this contract.

This work includes mobilization and demobilization required by the contract at the time of award. If additional mobilization and demobilization activities and costs are required during the performance of the contract as a result of changed, deleted, or added items of work for which the contractor is entitled to an adjustment in contract price, compensation for such costs will be included in the price adjustment for the item or items of work changed or added.

3. Payment

Payment will be made as the work proceeds, after presentation of paid invoices or documentation of direct costs by the contractor showing specific mobilization and demobilization costs and supporting evidence of the charges of suppliers, subcontractors, and others. When the total of such payments is less than the lump sum contract price, the balance remaining will be included in the final contract payment. Payment of the lump sum contract price for mobilization and demobilization will constitute full compensation for completion of the work.

Payment will not be made under this item for the purchase costs of materials having a residual value, the purchase costs of materials to be incorporated in the project, or the purchase costs of operating supplies.

4. Items of work and construction details

(See next page.)

4. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Bid Item 1, Mobilization and Demobilization

- (1) This item shall consist of mobilizing and demobilizing personnel and equipment in preparation to perform the work within the scope of this contract.
- (2) This item shall not include transportation of personnel, equipment and operating supplies between and within the work limit areas of this Contract.
- (3) Fences, which must be cut or removed for access, shall be repaired or replaced by the Contractor at his/her expense to equal or exceed the quality of fencing that was in place prior to cutting or removal.
- (4) Access shall be as shown on the drawings. If alternate routes are obtained by the Contractor, they must be approved by the Contracting Officer prior to use. All access routes shall be restored, by the Contractor, to a condition equal to or better than the condition prior to the commencement of work under this contract.
- (5) Payment will be as stated in Section 3, "Payment. Such payment will constitute full compensation for related Subsidiary Item, Traffic Control.

Construction Specification 9—Traffic Control

1. Scope

The work shall consist of establishing traffic control and maintaining safe, convenient use of public roads and rights-of-way.

2. Traffic and access

The contractor's operations shall cause no unnecessary inconvenience to the public. The public rights-of-way shall be maintained at all times unless interruption is authorized by proper local authority. Contractor's authorized closing or detour plans shall be provided to the engineer for approval.

Safe and adequate access shall be provided and maintained to all public protection devices and to all critical utility control locations. Facility access shall be continuous and unobstructed unless otherwise approved.

3. Storage of equipment and material in public streets

Construction materials and equipment shall not be stored or parked on public streets, roads, or highways. During any material or equipment loading or unloading activities that may temporarily interfere with traffic, an acceptable detour shall be provided for the duration of the activity. Any associated expense for this activity is the responsibility of the contractor.

Excavated material, including suitable material that is intended for adjacent trench backfill or other earth backfill as specified in section 5 of this specification, shall not be stored on public streets, roads, or highways that remain in service for the public. Any waiver of this requirement must be obtained from the proper local authority and approved by the engineer. All excess and unsuitable material shall be removed from the site as soon as possible. Any spillage shall be removed from roadways before they are used by the public.

4. Street closures, detours, and barricades

The contractor shall comply with the requirements of all applicable responsible units of government for closure of any street, road, or highway. The contractor shall provide the required barriers, guards, lights, signs, temporary bridges, and flaggers together with informing the public of any detours and construction hazards by the most suitable means available, such as local newspapers or radio stations. The contractor is also responsible for compliance with additional public safety requirements that may arise during construction. The contractor shall furnish, install, and, upon completion of the work, promptly remove all signs, warning devices, and other materials used in the performance of this work.

Unless otherwise specified, the contractor shall notify, in writing, the fire chief, police chief, county sheriff, state patrol, schools that operate school buses, or any other government official as may be appropriate no less than 7 days before closing, partly closing, or reopening any street, road, or highway.

Unless otherwise specified, the contractor shall furnish to the engineer a written plan showing the proposed method of signing, barricading for traffic control, and safety for street detours and closures.

All temporary detours will be maintained to ensure use of public rights-of-way is provided in a safe manner. This may include dust control, grading, and graveling as required in section 7 of this specification.

5. General and specific references

All signs, signals, barricades, use of flaggers, and other traffic control and public safety devices shall conform to the general requirements set forth in the Manual of Uniform Traffic Control Devices (MUTCD) and the latest edition of *Standard Highway Signs and Standard Alphabets for Highway Signs* and/or OSHA *Construction Industry Standards (29 CFR Part 1926), Subpart G, Signs, Signals, and Barricades* unless otherwise specified in section 7 of this specification.

6. Measurement and payment

For items of work for which specific lump sum prices are established in the contract, payment for the work is made at the contract lump sum price. Progress payments will be made based upon the percentage of estimated total time that traffic control will be required unless otherwise specified in section 7 of this specification. Payment will constitute full compensation for all flaggers, labor, materials, equipment, and all other items necessary and incidental to completion of the work.

Compensation for any item of work described in the contract, but not listed in the bid schedule will be included in the payment for the item of work to which it is made subsidiary. Such items and items to which they are made subsidiary are identified in section 7 of this specification.

7. Items of work and construction details

(See next page.)

7. Items of Work

Items of work to be performed in conformance with this specification are:

a. Subsidiary Item, Traffic Control

- (1) This item shall consist of providing the necessary traffic control devices (signs, signals, markings, personnel, etc.) where needed, to allow for the safe and expeditious movement of traffic through and adjacent to the construction area for the completion of this contract.
- (2) The contractor shall reopen all closed lanes at the end of the contractor's workday.
- (3) The contractor, prior to the start of construction operations, is to submit to the contacting officer, in writing, a proposed comprehensive traffic control plan, approved by the "Jefferson Parish Traffic Engineering Dept.". The Dept is located at 2100 Dickory Av., Jefferson, La. 70121, Phone 504-736-6530. The plan shall address, as a minimum, the following requirements:
 - (a) The design and application of all signals, pavement markings, channelizing devices, and warning signs shall conform to the *"Louisiana Manual on Uniform Traffic Control Devices"*, 1988 Edition, as revised, and the *"Work Area Traffic Control Handbook"*, of Jefferson Parish.
 - (b) Channelizing and delineation devices shall be used to mark all construction areas. These shall be Type II and/or Type III barricades, and/or barrels, all fully reflectional with lights.
 - (c) Any traffic control devices (signs, signals, markings) which exist as part of the normal pre-construction scheme, and that do not apply to an appropriate situation, or in the way of construction, shall be covered, removed, or relocated by the contractor.
 - (d) Any temporary devices not applying to an appropriate situation, or that are no longer required, shall be covered, removed, or relocated by the contractor.
 - (e) The roadway and all traffic control devices shall be restored to original conditions by the contractor.
 - (f) All excavations shall be covered, backfilled, or protected, (see "b", above) at night and when work is not in progress. Excavated pits, etc., shall be fully fenced or barricaded (see "b", above) to prevent access by pedestrians.
 - (g) All materials/machines shall be stored outside of the road surface, creating no sight distance problems, and fully delineated as in "b", above.
 - (h) If sections of roadway are totally closed, the contractor shall notify the sheriff's traffic division, 911 operators, the fire department, and any major traffic generators (i.e. schools, etc.). A three working day, minimum, advance notice will be required.
 - (i) On totally closed sections of the roadway, the contractor shall provide access for local traffic only.

- (j) Barrels used for channelizing and delineation devices as part of this traffic control device plan shall be weighted with sandbags and fully reflectional with lights.
 - (k) The contractor shall check traffic control devices on a daily basis as a minimum when beginning and ending the work day, to insure adherence to the plans and proper adequacy of devices for day and night visibility. On weekends, devices shall be checked a minimum once per day.
 - (l) Flagman and/or sheriff's control shall be provided as specified by the COTR.
 - (m) At the time of construction, any additional traffic control devices which may be required by the COTR, the Jefferson Parish Department of Public Works, or the contractor shall be provided by the contractor.
 - (n) Yellow, high visibility pennant barrier flagging (nylon rope with plastic pennants) shall be strung between Type II barricades and barrels/drums, only as directed by the COTR.
 - (o) This traffic control device plan indicates general traffic control devices to be used on this project. It is anticipated that conditions will vary depending on the phase under construction and that the arrangement of those devices will be reviewed on a daily basis. Should the contractor have any question as to the arrangement of those devices, the COTR shall be notified to make an inspection of the site.
 - (p) Contractor shall provide for the movement of pedestrians for the entire length of the contract. As much as possible, the contractor shall not obstruct existing sidewalks, thereby obstructing pedestrian movements. If existing sidewalks must be obstructed, the contractor shall provide for the movement of pedestrians by posting appropriate signing, such as, "Sidewalk Closed-Use Other Side of Street". Signing shall be reflectorized and lighted at night.
 - (q) Also, where trenches are excavated outside the roadway surface, the contractor shall provide each dwelling at least one accessible crossing of the backfilled trench area, for use by pedestrians.
 - (r) As much as possible, the contractor shall provide access to area businesses.
- (4) No separate payment will be made for this item. Compensation for Subsidiary Item, Traffic Control will be included in the payment for Bid Item 1, Mobilization and Demobilization.

Construction Specification 11—Removal of Water

1. Scope

The work consists of the removal of surface water and ground water as necessary to perform the construction required by the contract in accordance with the specifications. It shall include: (1) constructing, installing, building, and maintaining all necessary temporary water containment facilities, channels, and diversions; (2) furnishing, installing, and operating all necessary pumps, piping, and other facilities and equipment; and (3) removing all such temporary works and equipment after their intended function is no longer required.

2. Diverting surface water

The contractor shall install, maintain, and operate all cofferdams, channels, flumes, sumps, and all other temporary diversion and protective works needed to divert streamflow and other surface water through or around the construction site. Control of surface water shall be continuous during the period that damage to construction work could occur. Unless otherwise specified and/or approved, the diversion outlet shall be into the same drainageway that the water would have reached before being diverted.

The contractor shall furnish the contracting officer, in writing, a proposed plan for diverting surface water before beginning any construction activities for which a diversion is required, unless waived in section 8 of this specification. Acceptance of this plan or the waiving of the plan requirement will not relieve the contractor of the responsibilities related to this activity during the process of completing the work as specified.

3. Dewatering the construction site

Foundations, cutoff trenches, and all other parts of the construction site shall be dewatered and kept free of standing water and muddy conditions as necessary for the proper execution of the work. The contractor shall furnish, install, operate, and maintain all drains, sumps, pumps, casings, well points, and all other equipment required to properly dewater the site as specified. Dewatering systems that cause a loss of soil fines from the foundation areas will not be permitted.

The contractor shall furnish the contracting officer, in writing, a proposed plan for dewatering before commencing with any construction activity for which dewatering may be required, unless waived in section 8 of this specification. Acceptance of this plan or the waiving of the plan requirement will not relieve the contractor of the responsibilities for completing the specified work.

4. Dewatering borrow areas

The contractor shall maintain all borrow areas free of surface water or otherwise provide for timely and effective removal of surface and subsurface water that accumulates within the borrow area, unless waived in section 8 of this specification. Borrow material shall be processed as necessary to achieve proper and uniform moisture content at the time of placement.

If pumping to dewater borrow areas is included as a bid item of work in the bid schedule, each pump discharge pipe shall be equipped with a water meter. The meter shall be such that the measured quantity of water is accurate within 3 percent of the true quantity. The contractor shall provide necessary support to perform accuracy tests of the water meter when requested by the contracting officer.

5. Erosion and pollution control

Removal of water from the construction site, including the borrow areas, shall be accomplished so that erosion and the transporting of sediment and other pollutants are minimized. Dewatering activities shall be accomplished in a manner that the water table water quality is not altered. Pollution control activities

shall not conflict with the requirements of Construction Specification 5, Pollution Control, if it is a part of this contract.

6. Removal of temporary works

When temporary works are no longer needed, the contractor shall remove and return the area to a condition similar to that which existed before construction. Areas where temporary works were located shall be graded for sightly appearance with no obstruction to natural surface waterflows or the proper functioning and access to the works of improvement installed. The contractor shall exercise extreme care during the removal stages to minimize the loss of soil sediment and debris that was trapped during construction.

Pipes, casings, and any other material used to dewater the site shall be removed from temporary wells. The wells shall be filled to ground level with clean gravel or other suitable material approved by the contracting officer. The contractor shall exercise extreme care to prevent pollution of the ground water by these actions.

7. Measurement and payment

Method 1—Items of work listed in the bid schedule for removal of water, diverting surface water, and dewatering construction sites and borrow areas are paid for at the contract lump sum prices. Such payment will constitute full compensation for all labor, equipment, tools, and all other items necessary and incidental to the completion of the work.

Method 2—Items of work listed in the bid schedule for removal of water, diverting surface water, dewatering construction sites, and dewatering borrow areas are paid for at the contract lump sum prices. Such payment will constitute full compensation for furnishing, installing, operating, and maintaining the necessary trenches, drains, sumps, pumps, and piping and for all labor, equipment, tools, and all other items necessary and incidental to the completion of the work. The exception is that additional payment for pumping to dewater borrow areas and the removal of water will be made as described in the following paragraph.

If pumping to dewater borrow areas is a contract bid item, payment is made at the contract unit price, which shall be the price per 1,000 gallons shown in the bid schedule. Such payment will constitute full compensation for pumping only. Compensation for equipment and preparation and for other costs associated with pumping is included in the lump sum payment for removal of water or the lump sum payment for dewatering the borrow areas. Payment is made only for pumping that is necessary to dewater borrow areas that cannot be effectively drained by gravity or that must have the water table lowered to be usable as a suitable borrow source. Pumping for other purposes will not be included for payment under this item.

All Methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the contract line item to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 8 of this specification.

8. Items of work and construction details

8. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefor are:

a. Subsidiary Item, Removal of Water

- (1) The work shall consist of the removal of surface water and ground water as needed to perform the required construction in accordance with the specifications. It shall include, but not limited to, (1) building and maintaining all necessary temporary impounding works, channels, and diversions, (2) furnishing, installing and operating all necessary pumps, piping and other facilities and equipment, and (3) removing all such temporary works and equipment after they have served their purposes.
- (2) No dewatering plan will be required.
- (3) No separate payment will be made for this item. Compensation for Subsidiary Item, "Removal of Water" will be included in the payment for Bid Item 5, Rockfill.

Construction Specification 13—Piling

1. Scope

The work consists of furnishing and installing the specified kinds and types of piles at the locations shown on the drawings.

2. Material

Piles shall conform to the requirements of following material specifications as appropriate to the kinds of piles specified. For piles of materials other than those listed, the material requirements outlined in section 14 of this specification shall apply.

511—Steel Piles

512—Wood Piles

513—Precast Concrete Piles

514—Cast-in-Place Concrete Piles

3. Site preparation

All excavation within the area to be occupied by bearing piles shall be completed before the piles are driven.

4. Protection of pile heads

The heads of all piles shall be protected during driving by suitable caps, rings, heads, blocks, mandrels, and other devices.

The heads of timber piles shall be fitted into a steel head block or fitted with heavy steel or wrought iron rings or wire wrapping.

The heads of steel piles shall be cut square and fitted with a steel driving cap.

The heads of precast concrete piles and casings shall be fitted into cushion type drive caps having a rope or other suitable cushion next to the pile head and fitting into a casting that in turn supports a timber shock block.

Driving heads, mandrels, and other devices shall be provided by the contractor as needed for special types of piles and shall conform to the recommendations of the pile manufacturer.

5. Piles, general

The contractor shall notify the engineer before pile driving operation commences. Such notice shall be far enough in advance, a minimum of 24 hours, to provide the engineer adequate time to be present for the driving operations. Piles shall be driven only in the presence of the engineer or authorized representative.

The determination of piling order lengths shall be the contractor's responsibility unless otherwise specified.

Unless otherwise approved, piles shall be driven with steam, air, diesel powered hammers or a combination of hammers, or by vibration or water jets. Water jets may be used only when specifically authorized by the engineer. Where jetting is authorized, the jets shall be withdrawn before the specified depth or bearing capacity is obtained and the piles shall be driven with the hammer to the final penetration.

When drop hammers are permitted, the height of drop shall not be more than 8 feet for concrete piles or 12 feet for steel and timber piles, unless otherwise specified.

The driving of piling with followers shall be allowed only when expressly approved by the engineer.

Piles shall not be driven within 20 feet of concrete less than 7 days after placement, including concrete placed in cast-in-place piles with or without predriven shells or casings.

The contractor shall not attempt to drive piles beyond the point of refusal, as indicated by excessive bouncing of the hammer or kicking of the pile.

6. Bearing piles

Bearing piles shall be driven to the position, line, and batter specified on the drawings. Each pile shall be driven continuously and without interruption to the specified depth or until the specified bearing capacity is obtained. Deviation from this procedure is permitted only when interruption of driving is caused by conditions that could not reasonably be anticipated.

When a diesel hammer is used, it shall be operated at full throttle when blows are counted for determination of bearing capacity except that throttle adjustments shall be made as necessary to prevent the non-striking parts of the hammer from rising from the pile on the ram upstroke.

7. Sheet piles

The piling shall be driven in a manner that ensures perfect interlocking throughout the entire length of each pile. The piles shall be held in proper alignment during driving by assembling frames or other suitable temporary guide structures. Temporary guide structures shall be removed when they have served their purpose.

Anytime the forward edge of the sheet pile wall is found to be out of correct alignment,

- a. The piling already assembled and partly driven shall be driven to the required depth.
- b. Taper piles shall then be driven to bring the forward edge into correct alignment before additional regular piling is assembled and driven. The maximum permissible taper in a single pile shall be 0.25 inch per foot of length.

8. Estimating bearing capacity

When load tests are not required, the bearing capacity of each pile shall be estimated using one of the following formulas, as appropriate:

Gravity hammers:

$$R = \frac{2WH}{S + 1}$$

Single-acting steam or air hammers and diesel hammers having unrestricted rebound of the ram:

$$R = \frac{2WH}{S + 0.1}$$

Double-acting steam or air hammers and diesel hammers having enclosed rams:

$$R = \frac{2H(W + AP)}{S + 0.1} \quad \text{or} \quad R = \frac{2E}{S + 0.1}$$

where:

R = safe bearing capacity, in pounds

W = weight of striking parts of hammer, in pounds

- H = height of fall, in feet
A = area of piston, in square inches
P = pressure of steam, air, or other gas exerted on the hammer piston or ram, in pounds per square inch
E = the manufacturer's rating for foot-pounds of energy developed by double-acting steam or air hammers, or 90 percent of the average equivalent energy developed by diesel hammers having enclosed rams as evaluated by gauge and chart readings, in foot-pounds
S = average penetration for the last 5 to 10 blows of a gravity hammer or the last 10 to 20 blows for steam, air, or diesel powered hammers, in inches per blow

These formulas are applicable when:

- The hammer has a free fall.
- The head of the pile is not crushed.
- The penetration is reasonably quick and uniform.
- There is no sensible bounce after the blow.
- A follower is not used.

Twice the height of the bounce shall be deducted from **H** to determine its value in the formula.

If case water jets are used in conjunction with the driving, these formulas are used to determine the bearing power from the results of driving after the jets have been removed.

9. Load tests

When load tests are specified, the test loads shall be applied gradually, without impact, and in a manner that no lateral forces are applied to the pile. Load testing shall not be started until 24 hours after driving of the test pile is completed unless otherwise specified in section 14 of this specification. Except as otherwise specified, load tests shall be performed according to the following procedures.

The total test load shall be twice the specified working load and shall be applied to the pile in increments equal to 25 percent of the working load. Settlement of the top of the pile shall be measured to an accuracy of 0.01 inch before and after the application of each load increment and at 2, 4, 8, 15, 30, and 60 minutes after, and then every 2 hours until the next load increment is applied. Additional load shall not be applied until the rate of settlement is less than 0.01 inch in 1 hour.

The total test load shall remain on the pile for a minimum of 24 hours. Settlement shall be measured at 6-hour intervals during this period and at the end of the period, at least twice during removal of the load, and immediately after all of the test load is removed. The net settlement shall be measured about 24 hours after the total load has been removed.

If settlement continues in excess of 0.01 inch per hour under less than the total test load, no additional load shall be applied. However, the load that has been applied shall remain on the pile a minimum of 24 hours, and settlement measurements while the load is on the pile and during and after removal of the load shall be made as if it were the total test load.

10. Cutting off piles

The contractor shall cut the piles at the specified elevations. The length of pile cut off shall be sufficient to permit the removal of all damaged material. Steel shells or concrete casings for cast-in-place concrete piles shall be cut off at the specified elevation before being filled with concrete.

Steel bearing piles shall be cut off in clean, straight lines as shown on the drawings. Any irregularities shall be leveled off with deposits of weld metal or by grinding before placement of bearing caps.

Precast concrete piles and concrete casings shall be cut off in a manner that prevents damage to the rest of the pile or casing or to the projecting reinforcement required for connecting the piles to the structure.

Timber piles that are to be capped shall be accurately cut off so that true bearing is obtained on every pile without the use of shims.

11. Defective piles

Any pile damaged in driving, driven out of proper location, driven below the specified cutoff elevation, or inaccurately cut off shall be corrected by one of the following methods, as approved by the engineer:

- a. The defective pile shall be pulled and replaced or re-driven.
- b. A new pile shall be driven adjacent to the defective pile.
- c. The defective pile shall be spliced or built up or a sufficient part of the footing shall be extended to properly embed the pile.

Pile shells abandoned in place after driving shall be filled with concrete or sand-cement grout as appropriate to the conditions that are present.

All piles pushed up by the driving of adjacent piles or by any other cause shall be re-driven to final grade.

Any sheet pile ruptured in the interlock or otherwise damaged during driving shall be pulled and replaced.

12. Correcting surface heave

Any excess material resulting from displacement of earth by pile driving shall be removed. Materials disturbed by pile driving shall be conditioned and compacted to a minimum density equal to adjacent undisturbed material.

13. Measurement and payment

Method 1—For items of work for which specific unit prices are established in the contract, each type, kind, and length of pile driven in place is counted. Payment for furnishing and driving each type, kind, and length of pile is made at the contract unit price. Such payment will constitute full compensation for all labor, equipment, materials, and all other items necessary and incidental to the completion of the work.

Method 2—For items of work for which specific unit prices are established in the contract, each type, kind, and length of pile furnished, accepted, and stockpiled in good condition at the site of the work is counted. Payment for furnishing each type, kind, and length of pile is made at the contract unit price. Payment for driving each type and kind of pile is made at the contract unit price. Such payment will constitute full compensation for all labor, equipment, materials, and all other items necessary and incidental to the completion of the work.

Method 3—For items of work for which specific unit prices are established in the contract, the length of each type and kind of pile driven is computed to the nearest foot as the difference between the measured length of pile before driving and measured length of pile cut off after driving. Payment for furnishing and driving each type and kind of pile is made at the contract unit price. Such payment will constitute full payment for all labor, equipment, materials, and other items necessary and incidental to the completion of the work.

Method 4—For items of work for which specific unit prices are established in the contract, the area of sheet pile walls, acceptably placed in accordance and within the neat lines shown on the drawings, is computed to the nearest square foot. Payment is made at the contract unit price for each type, kind, and

weight of piling. Such payment will constitute full payment for all labor, equipment, materials, and other items necessary and incidental to the completion of the work.

All Methods—The following provisions apply to all methods of measurement and payment:

The measurement of the number of linear feet of piles (or number of piles) furnished and the number of piles driven shall include test and tension piles specified in the contract. Piles furnished and driven at the option of the contractor are not included. No payment is made for furnishing or driving pile, including test piles, to replace piles lost or damaged before the completion of the contract while in stockpile or during handling and driving.

When load tests are specified, payment for each test is made at the contract unit price per test. Such payment will constitute full compensation for all labor, equipment, materials, and all other items necessary and incidental to perform the test, except furnishing and driving piling.

When splices are specified, payment for each splice is made at the contract unit price. Such payment shall constitute full compensation for labor, equipment, materials, and all other items necessary and incidental to the completion of the work.

Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 14 of this specification.

14. Items of work and construction details

14. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Bid Item 7, Steel Sheetpile Toewall

- (1) This item shall consist of furnishing and driving all steel sheet piling required, to construct the toe walls to the lines and grades as shown on the drawings.
- (2) All piling shall conform to Material Specification 511, Steel Piles.
- (3) All steel piling shall conform to ASTM A 328 or A 572, Grade 50.
- (4) The first pile shall be driven on the extreme left or right side of the repair area and progress by driving the female (socket) end over the male end of the previous pile.
- (5) The minimum section modulus and associated thickness of each pile shall be as shown on the drawings.
- (6) Only "normal" configuration (as shown on the drawings) will be permitted when driving sheetpiling.
- (7) Drop gravity hammers will be permitted.
- (8) Section 13, Measurement and Payment of this specification is deleted in its entirety and replaced as follows:

Payment will be made at the specified lump sum price as established in the contract. Such payment shall be considered full compensation for all material, labor, equipment, tools, vegetation and other items necessary and incidental to the completion of the work.

Construction Specification 21—Excavation

1. Scope

The work shall consist of the excavation required by the drawings and specifications and disposal of the excavated materials.

2. Classification

Excavation is classified as **common excavation**, **rock excavation**, or **unclassified excavation** in accordance with the following definitions.

Common excavation is defined as the excavation of all materials that can be excavated, transported, and unloaded using heavy ripping equipment and wheel tractor-scrapers with pusher tractors or that can be excavated and dumped into place or loaded onto hauling equipment by excavators having a rated capacity of one cubic yard or larger and equipped with attachments (shovel, bucket, backhoe, dragline, or clam shell) appropriate to the material type, character, and nature of the materials.

Rock excavation is defined as the excavation of all hard, compacted, or cemented materials that require blasting or the use of ripping and excavating equipment larger than defined for common excavation. The excavation and removal of isolated boulders or rock fragments larger than 1 cubic yard encountered in materials otherwise conforming to the definition of common excavation shall be classified as rock excavation. The presence of isolated boulders or rock fragments larger than 1 cubic yard is not in itself sufficient cause to change the classification of the surrounding material.

For the purpose of these classifications, the following definitions shall apply:

Heavy ripping equipment is a rear-mounted, heavy duty, single-tooth, ripping attachment mounted on a track type tractor having a power rating of at least 250 flywheel horsepower unless otherwise specified in section 10.

Wheel tractor-scraper is a self-loading (not elevating) and unloading scraper having a struck bowl capacity of at least 12 cubic yards.

Pusher tractor is a track type tractor having a power rating of at least 250 flywheel horsepower equipped with appropriate attachments.

Unclassified excavation is defined as the excavation of all materials encountered, including rock materials, regardless of their nature or the manner in which they are removed.

3. Blasting

The transportation, handling, storage, and use of dynamite and other explosives shall be directed and supervised by a person(s) of proven experience and ability who is authorized and qualified to conduct blasting operations.

Blasting shall be done in a manner as to prevent damage to the work or unnecessary fracturing of the underlying rock materials and shall conform to any special requirements in section 10 of this specification. When specified in section 10, the contractor shall furnish the engineer, in writing, a blasting plan before blasting operations begin.

4. Use of excavated material

Method 1—To the extent they are needed, all suitable material from the specified excavations shall be used in the construction of required permanent earthfill or rockfill. The suitability of material for specific purposes is determined by the engineer. The contractor shall not waste or otherwise dispose of suitable excavated material.

Method 2—Suitable material from the specified excavations may be used in the construction of required earthfill or rockfill. The suitability of material for specific purposes is determined by the engineer.

5. Disposal of waste materials

Method 1—All surplus or unsuitable excavated materials are designated as waste and shall be disposed of at the locations shown on the drawings.

Method 2—All surplus or unsuitable excavated materials are designated as waste and shall be disposed of by the contractor at sites of his own choosing away from the site of the work. The disposal shall be in an environmentally acceptable manner that does not violate local rules and regulations.

6. Excavation limits

Excavations shall comply with OSHA Construction Industry Standards (29CFR Part 1926) Subpart P, Excavations, Trenching, and Shoring. All excavations shall be completed and maintained in a safe and stable condition throughout the total construction phase. Structure and trench excavations shall be completed to the specified elevations and to the length and width required to safely install, adjust, and remove any forms, bracing, or supports necessary for the installation of the work. Excavations outside the lines and limits shown on the drawings or specified herein required to meet safety requirements shall be the responsibility of the contractor in constructing and maintaining a safe and stable excavation.

7. Borrow excavation

When the quantities of suitable material obtained from specified excavations are insufficient to construct the specified earthfills and earth backfills, additional material shall be obtained from the designated borrow areas. The extent and depth of borrow pits within the limits of the designated borrow areas shall be as specified in section 10 or as approved by the engineer.

Borrow pits shall be excavated and finally dressed to blend with the existing topography and sloped to prevent ponding and to provide drainage.

8. Overexcavation

Excavation in rock beyond the specified lines and grades shall be corrected by filling the resulting voids with portland cement concrete made of materials and mix proportions approved by the engineer. Concrete that will be exposed to the atmosphere when construction is completed shall meet the requirements of concrete selected for use under Construction Specification 31, Concrete for Major Structures, or 32, Structure Concrete, as appropriate.

Concrete that will be permanently covered shall contain not less than five bags of cement per cubic yard. The concrete shall be placed and cured as specified by the engineer.

Excavation in earth beyond the specified lines and grades shall be corrected by filling the resulting voids with approved, compacted earthfill. The exception to this is that if the earth is to become the subgrade for riprap, rockfill, sand or gravel bedding, or drainfill, the voids may be filled with material conforming to the specifications for the riprap, rockfill, bedding, or drainfill. Before correcting an overexcavation condition, the contractor shall review the planned corrective action with the engineer and obtain approval of the corrective measures.

9. Measurement and payment

For items of work for which specific unit prices are established in the contract, the volume of each type and class of excavation within the specified pay limits is measured and computed to the nearest cubic yard by the method of average cross-sectional end areas or by methods outlined in section 10 of this specification. Regardless of quantities excavated, the measurement for payment is made to the specified pay limits except that excavation outside the specified lines and grades directed by the engineer to remove unsuitable material is included. Excavation required because unsuitable conditions result from the contractor's improper construction operations, as determined by the engineer, is not included for measurement and payment.

Method 1—The pay limits shall be as designated on the drawings.

Method 2—The pay limits shall be defined as follows:

- a. The upper limit shall be the original ground surface as it existed before the start of construction operations except that where excavation is performed within areas designated for previous excavation or earthfill, the upper limit shall be the modified ground surface resulting from the specified previous excavation or earthfill.
- b. The lower and lateral limits shall be the neat lines and grades shown on the drawings.

Method 3 —The pay limits shall be defined as follows:

- a. The upper limit shall be the original ground surface as it existed before the start of construction operations except that where excavation is performed within areas designated for previous excavation or earthfill, the upper limit shall be the modified ground surface resulting from the specified previous excavation or earthfill.
- b. The lower and lateral limits shall be the true surface of the completed excavation as directed by the engineer.

Method 4—The pay limits shall be defined as follows:

- a. The upper limit shall be the original ground surface as it existed before the start of construction operations except that where excavation is performed within areas designated for previous excavation or earthfill, the upper limit shall be the modified ground surface resulting from the specified previous excavation or earthfill.
- b. The lower limit shall be at the bottom surface of the proposed structure.
- c. The lateral limits shall be 18 inches outside of the outside surface of the proposed structure or shall be vertical planes 18 inches outside of and parallel to the footings, whichever gives the larger pay quantity, except as provided in d below.
- d. For trapezoidal channel linings or similar structures that are to be supported upon the sides of the excavation without intervening forms, the lateral limits shall be at the underside of the proposed lining or structure.
- e. For the purposes of the definitions in b, c, and d, above, any specified bedding or drainfill directly beneath or beside the structure will be considered to be a part of the structure.

All methods—The following provisions apply to all methods of measurement and payment.

Payment for each type and class of excavation is made at the contract unit price for that type and class of excavation. Such payment will constitute full compensation for all labor, materials, equipment, and all other items necessary and incidental to the performance of the work except that extra payment for backfilling overexcavation will be made in accordance with the following provisions.

Payment for backfilling overexcavation, as specified in section 8 of this specification, is made only if the excavation outside specified lines and grades is directed by the engineer to remove unsuitable material and if the unsuitable condition is not a result of the contractor's improper construction operations as determined by the engineer.

Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 10 of this specification.

10. Items of work and construction details

10. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefor are:

a. Bid Item 3, Excavation

- (1) This item shall consist of all excavation required to perform the work as shown in the plans, as called for in the specifications and as staked in the field. Excavation shall be classified as *Common Excavation*.
- (2) If the contractor elects to perform any of the excavation in the wet (under normal water level conditions) he will assure that the final excavated surface shall be uniform and free from any abrupt changes in grade. The lines and grades shown on the drawings or staked in the field shall be adhered to.
- (3) All unconsolidated and undesirable loose material, as determined by the COTR, shall be removed from the damaged slope area. The final surface of the excavated area shall be undisturbed soil and have a slope no steeper than one horizontal to one vertical (1:1) or as shown on the drawings.
- (4) Use of excavated material shall be by Method 2 with the following addition; “Only the excavated material that meets the requirements of Specification 23, “Earthfill” and only the amount necessary to complete the earthfill, requirements shall be stockpiled on the construction site for later use”.
- (5) Disposal of waste material shall be by Method 1 with the following addition; “All excess or undesirable excavated material shall be loaded and hauled to the “Soil Disposal Site” shown on the drawings. If approved by the COTR excavated soil maybe used at the construction site to fill holes along the channel berm within the work limits. Any soil used in this manner will be blade dressed and left in a smooth and neat condition”.
- (6) The completed excavation shall not remain exposed for a period of time exceeding 24 hours prior to the start of the placement of the geotextile, drainfill, rockfill, rock riprap and/or earthfill.
- (7) Section 9, “Measurement and Payment”, of this specification is deleted in its entirety and is replaced as follows:

Payment shall be made at the lump sum price in the contract and such payment shall be considered as full compensation for all labor, materials, equipment, and all other items necessary and incidental to the performance of the work. Such payment will constitute full compensation for related work with the exception that extra payment for backfilling overexcavation will be made in accordance with the following provisions:

Payment for backfilling overexcavation is made only if the excavation outside specified lines and grades is directed by the COTR to remove unsuitable material and if the unsuitable condition is not a result of the contractor’s improper construction operations as determined by the COTR.

Such payment will constitute full compensation for related Subsidiary Items, “Clearing and Grubbing”, “Structure Removal”.

Construction Specification 23—Earthfill

1. Scope

The work consists of the construction of earth embankments, other earthfills, and earth backfills required by the drawings and specifications.

Earthfill is composed of natural earth materials that can be placed and compacted by construction equipment operated in a conventional manner.

Earth backfill is composed of natural earth material placed and compacted in confined spaces or adjacent to structures (including pipes) by hand tamping, manually directed power tampers or vibrating plates, or their equivalent.

2. Material

All fill material shall be obtained from required excavations and designated borrow areas. The selection, blending, routing, and disposition of material in the various fills shall be subject to approval by the engineer.

Fill materials shall contain no frozen soil, sod, brush, roots, or other perishable material. Rock particles larger than the maximum size specified for each type of fill shall be removed prior to compaction of the fill.

The types of material used in the various fills shall be as listed and described in the specifications and drawings.

3. Foundation preparation

Foundations for earthfill shall be stripped to remove vegetation and other unsuitable material or shall be excavated as specified.

Except as otherwise specified, earth foundation surfaces shall be graded to remove surface irregularities and shall be scarified parallel to the axis of the fill or otherwise acceptably scored and loosened to a minimum depth of 2 inches. The moisture content of the loosened material shall be controlled as specified for the earthfill, and the surface material of the foundation shall be compacted and bonded with the first layer of earthfill as specified for subsequent layers of earthfill.

Earth abutment surfaces shall be free of loose, uncompacted earth in excess of 2 inches in depth normal to the slope and shall be at such a moisture content that the earthfill can be compacted against them to produce a good bond between the fill and the abutments.

Rock foundation and abutment surfaces shall be cleared of all loose material by hand or other effective means and shall be free of standing water when fill is placed upon them. Occasional rock outcrops in earth foundations for earthfill, except in dams and other structures designed to restrain the movement of water, shall not require special treatment if they do not interfere with compaction of the foundation and initial layers of the fill or the bond between the foundation and the fill.

Foundation and abutment surfaces shall be no steeper than one horizontal to one vertical unless otherwise specified. Test pits or other cavities shall be filled with compacted earthfill conforming to the specifications for the earthfill to be placed upon the foundation.

4. Placement

Earthfill shall not be placed until the required excavation and foundation preparation have been completed and the foundation has been inspected and approved by the engineer. Earthfill shall not be placed upon a frozen surface nor shall snow, ice, or frozen material be incorporated in the earthfill matrix.

Earthfill shall be placed in approximately horizontal layers. The thickness of each layer before compaction shall not exceed the maximum thickness specified in section 10 or shown on the drawings. Materials placed by dumping in piles or windrows shall be spread uniformly to not more than the specified thickness before being compacted.

Hand compacted earth backfill shall be placed in layers whose thickness before compaction does not exceed the maximum thickness specified for layers of earth backfill compacted by manually directed power tampers.

Earth backfill shall be placed in a manner that prevents damage to the structures and allows the structures to assume the loads from the earth backfill gradually and uniformly. The height of the earth backfill adjacent to a structure shall be increased at approximately the same rate on all sides of the structure.

Earthfill and earth backfill in dams, levees, and other structures designed to restrain the movement of water shall be placed to meet the following additional requirements:

- (a) The distribution of materials throughout each zone shall be essentially uniform, and the earthfill shall be free from lenses, pockets, streaks, or layers of material differing substantially in texture, moisture content, or gradation from the surrounding material. Zone earthfills shall be constructed concurrently unless otherwise specified.
- (b) If the surface of any layer becomes too hard and smooth for proper bond with the succeeding layer, it shall be scarified parallel to the axis of the fill to a depth of not less than 2 inches before the next layer is placed.
- (c) The top surface of embankments shall be maintained approximately level during construction with two exceptions: A crown or cross-slope of about 2 percent shall be maintained to ensure effective drainage, or as otherwise specified for drainfill or sectional zones.
- (d) Dam embankments shall be constructed in continuous layers from abutment to abutment except where openings to facilitate construction or to allow the passage of streamflow during construction are specifically authorized in the contract.
- (e) Embankments built at different levels as described under (c) or (d) above shall be constructed so that the slope of the bonding surfaces between embankment in place and embankment to be placed is not steeper than 3 feet horizontal to 1 foot vertical. The bonding surface of the embankment in place shall be stripped of all material not meeting the requirements of this specification and shall be scarified, moistened, and recompactd when the new earthfill is placed against it. This ensures a good bond with the new earthfill and obtains the specified moisture content and density at the contact of the in-place and new earthfills.

5. Control of moisture content

During placement and compaction of earthfill and earth backfill, the moisture content of the material being placed shall be maintained within the specified range.

The application of water to the earthfill material shall be accomplished at the borrow areas insofar as practicable. Water may be applied by sprinkling the material after placement on the earthfill, if necessary. Uniform moisture distribution shall be obtained by disking.

Material that is too wet when deposited on the earthfill shall either be removed or be dried to the specified moisture content prior to compaction.

If the top surface of the preceding layer of compacted earthfill or a foundation or abutment surface in the zone of contact with the earthfill becomes too dry to permit suitable bond, it shall either be removed or scarified and moistened by sprinkling to an acceptable moisture content before placement of the next layer of earthfill.

6. Compaction

Earthfill— Earthfill shall be compacted according to the following requirements for the class of compaction specified:

Class A compaction—Each layer of earthfill shall be compacted as necessary to provide the density of the earthfill matrix not less than the minimum density specified in Section 10 or identified on the drawings. The earthfill matrix is defined as the portion of the earthfill material finer than the maximum particle size used in the compaction test method specified.

Class B compaction—Each layer of earthfill shall be compacted to a mass density not less than the minimum density specified.

Class C compaction—Each layer of earthfill shall be compacted by the specified number of passes of the type and weight of roller or other equipment specified or by an approved equivalent method. Each pass shall consist of at least one passage of the roller wheel or drum over the entire surface of the layer.

Earth backfill— Earth backfill adjacent to structures shall be compacted to a density equivalent to that of the surrounding in-place earth material or adjacent required earthfill or earth backfill. Compaction shall be accomplished by hand tamping or manually directed power tampers, plate vibrators, walk-behind, miniature, or self-propelled rollers. Unless otherwise specified heavy equipment including backhoe mounted power tampers or vibrating compactors and manually directed vibrating rollers shall not be operated within 2 feet of any structure. Towed or self-propelled vibrating rollers shall not be operated within 5 feet of any structure. Compaction by means of drop weights operating from a crane or hoist is not permitted.

The passage of heavy equipment will not be allowed:

- Over cast-in-place conduits within 14-days after placement of the concrete
- Over cradled or bedded precast conduits within 7 days after placement of the concrete cradle or bedding
- Over any type of conduit until the backfill has been placed above the top surface of the structure to a height equal to one-half the clear span width of the structure or pipe or 2 feet, whichever is greater, except as may be specified in section 10.

Compacting of earth backfill adjacent to structures shall not be started until the concrete has attained the strength specified in section 10 for this purpose. The strength is determined by compression testing of test cylinders cast by the contractor's quality control personnel for this purpose and cured at the work site in the manner specified in ASTM C 31 for determining when a structure may be put into service.

When the required strength of the concrete is not specified as described above, compaction of earth backfill adjacent to structures shall not be started until the following time intervals have elapsed after placement of the concrete.

Structure	Time interval (days)
Vertical or near-vertical walls with earth loading on one side only	14
Walls backfilled on both sides simultaneously	7
Conduits and spillway risers, cast-in-place (with inside forms in place)	7
Conduits and spillway risers, cast-in-place (inside forms removed)	14
Conduits, pre-cast, cradled	2
Conduits, pre-cast, bedded	1
Cantilever outlet bents (backfilled both sides simultaneously)	3

7. Reworking or removal and replacement of defective earthfill

Earthfill placed at densities lower than the specified minimum density or at moisture contents outside the specified acceptable range of moisture content or otherwise not conforming to the requirements of the specifications shall be reworked to meet the requirements or removed and replaced by acceptable earthfill. The replacement earthfill and the foundation, abutment, and earthfill surfaces upon which it is placed shall conform to all requirements of this specification for foundation preparation, approval, placement, moisture control, and compaction.

8. Testing

During the course of the work, the engineer will perform quality assurance tests required to identify material; determine compaction characteristics; determine moisture content; and determine density of earthfill in place. Tests performed by the engineer will be used to verify that the earthfills conform to contract requirements of the specifications and not as a replacement for the contractor's quality control program.

Densities of earthfill requiring Class A compaction will be determined in accordance with ASTM D 1556, D 2167, D 2922, or D 2937 except that the volume and moist weight of included rock particles larger than those used in the compaction test method specified for the type of fill will be determined and deducted from the volume and moist weight of the total sample before computation of density or, if using the nuclear gauge, added to the specified density to bring it to the measure of equivalent composition for comparison (see ASTM D 4718). The density so computed is used to determine the percent compaction of the earthfill matrix. Unless otherwise specified, moisture content is determined by one of the following methods: ASTM D 2216, D 3017, D 4643, D 4944, or D 4959.

9. Measurement and payment

For items of work for which specific unit prices are established in the contract, the volume of each type and compaction class of earthfill and earth backfill within the specified zone boundaries and pay limits is measured and computed to the nearest cubic yard by the method of average cross-sectional end areas. Unless otherwise specified in section 10, no deduction in volume is made for embedded items, such as, but not limited to, conduits, inlet structures, outlet structures, embankment drains, sand diaphragm and outlet, and their appurtenances.

The pay limits shall be as defined below, with the further provision that earthfill required to fill voids resulting from overexcavation of the foundation, outside the specified lines and grades, will be included in the measurement for payment only under the following conditions:

- Where such overexcavation is directed by the engineer to remove unsuitable material, and
- Where the unsuitable condition is not a result of the contractor's improper construction operations as determined by the engineer.

Earthfill beyond the specified lines and grades to backfill excavation required for compliance with OSHA requirements will be considered subsidiary to the earthfill bid item(s).

Method 1—The pay limits shall be as designated on the drawings.

Method 2—The pay limits shall be the measured surface of the foundation when approved for placement of the earthfill and the specified neat lines of the earthfill surface.

Method 3—The pay limits shall be the measured surface of the foundation when approved for placement of the earthfill and the measured surface of the completed earthfill.

Method 4—The pay limits shall be the specified pay limits for excavation and the specified neat lines of the earthfill surface.

Method 5—The pay limits shall be the specified pay limits for excavation and the measured surface of the completed earthfill.

Method 6—Payment for each type and compaction class of earthfill and earth backfill is made at the contract unit price for that type and compaction class of earthfill. Such payment will constitute full compensation for all labor, material, equipment, and all other items necessary and incidental to the performance of the work.

Method 7—Payment for each type and compaction class of earthfill and earth backfill is made at the contract unit price for that type and compaction class of earthfill. Such payment will constitute full compensation for all labor, material, equipment, and all other items necessary and incidental to the performance of the work except furnishing, transporting, and applying water to the foundation and earthfill material. Water applied to the foundation and earthfill material is measured and payment made as specified in Construction Specification 10.

All methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 10 of this specification.

10. Items of work and construction details

10. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Bid Item 4, Earthfill, Topsoil

- (1) This item shall consist of all work necessary to furnish, haul, place and shape the necessary earthfill, topsoil needed to complete the channel slope repair as shown on the drawings and specified herein.
- (2) When on-site earthfill, topsoil is not sufficient to complete the work, or when existing earthfill, topsoil is deemed unsuitable by the COTR, the contractor shall provide earthfill, topsoil from off-site borrow areas. Earthfill, topsoil, delivered to the site, shall be friable **surface** soil reasonably free of grass, roots, weeds, sticks, stones or other foreign material. It shall be classified as SC or CL on the Unified Soil Classification System. **All earthfill, topsoil shall be approved by the COTR before placement.** All rejected earthfill, topsoil will be removed from the construction site at the contractor's expense.
- (3) Compaction shall be Class C in accordance with Section 6 of this specification with the following additions; “Once the surface of the rock riprap, rockfill, or graded soil surface, has been approved by the COTR, the earthfill, topsoil will be spread in uniform layers, not to exceed six (6) inches, and lightly bucket compacted, to ensure a homogeneous mass, to the lines and grade shown on the drawings. Care will be taken in the placement of the earthfill, topsoil so as not to displace the rockfill or aggregate material. The finished surface of the earthfill, topsoil shall have a smooth surface free of clods”.
- (4) The moisture content of the earthfill, topsoil shall be homogeneous and shall be maintained at a level which will:
 - a. Prevent bulking or dilatant behavior of the material under the action of the hauling or placing equipment. Dilatant behavior is exhibited when a soil sample is shaken and the surface shines due to the movement of free water.
 - b. Prevent adherence of the material to the hauling or placing equipment.
 - c. Ensure the crushing and blending of the soil clods into a homogenous mass.
 - d. Allow a sample to be hand molded and will form a ball that does not readily separate and does not ooze through the fingers.
- (5) Section 9, “Measurement and Payment”, of this specification is deleted in its entirety and is replaced as follows:

Payment shall be made at the lump sum price in the contract and such payment shall be considered as full compensation for all material, labor, equipment, tools and other items necessary and incidental to the completion of the work as described in this contract.

Construction Specification 25—Rockfill

1. Scope

The work consists of the construction of rockfill zones of embankments and other rockfills required by the drawings and specifications, including bedding where specified.

2. Material

Material for rockfill and bedding shall be obtained from the specified sources unless otherwise specified in section 10 of this specification. The material shall be excavated, selected, processed, and handled as necessary to conform to the specified gradation requirements.

3. Foundation preparation

Foundations for rockfill shall be stripped to remove vegetation and other unsuitable material or shall be excavated as specified.

Except as otherwise specified, earth foundation surfaces shall be graded to remove surface irregularities, and test pits or other cavities shall be filled with compacted earthfill of approximately the same kind and density as the adjacent foundation material.

Rock foundation surfaces shall be cleared of all loose material not conforming to the specifications for the rockfill.

Abutments for rockfill zones of embankments shall be prepared as specified above for foundations.

Rockfill and/or bedding shall not be placed until the foundation preparation is completed and the foundation and excavations have been inspected and approved.

4. Bedding

When a bedding layer beneath rockfill is specified, the bedding material shall be spread uniformly on the prepared subgrade surfaces to the depth indicated. Compaction of the bedding material shall be as specified in section 10 of this specification.

5. Placement

Method 1—The rock shall be dumped and spread into position in approximately horizontal layers not to exceed 3 feet in thickness. It shall be placed to produce a reasonably homogeneous stable fill that contains no segregated pockets of large or small fragments or large unfilled spaces caused by bridging of the larger rock fragments.

Method 2—The rock shall be dumped and spread into position in approximately horizontal layers not to exceed 3 feet in thickness. The rock shall be placed so that the completed fill shall be graded with the smaller rock fragments placed in the inner portion of the embankment and the larger rock fragments placed on the outer slopes. Rock shall be placed to produce a stable fill that contains no large unfilled spaces caused by bridging of the larger fraction.

6. Control of moisture

The moisture content of rockfill material shall be controlled as specified in section 10 of this specification. When the addition of water is required, it shall be applied in a manner to avoid excessive wetting of adjacent earthfill. Except as specified in section 10 of this specification, control of the moisture content is not required.

The moisture content of the bedding material shall be controlled to ensure that bulking of the sand materials does not occur during compaction operations.

7. Compaction of rockfill

Rockfill shall be compacted as described below for the class of compaction specified or by an approved equivalent method.

Class I compaction—Each layer of fill shall be compacted by at least four passes over the entire surface with a steel-drum vibrating roller that weighs at least 5 tons and exerting a vertical vibrating force of not less than 20,000 pounds at a frequency not less than 1,200 times per minute.

Class II compaction—Each layer of fill shall be compacted by at least four passes over the entire surface by a track of a crawler-type tractor weighing at least 20 tons.

Class III compaction—No compaction is required beyond that resulting from the placing and spreading operations.

Heavy equipment shall not be operated within 2 feet of any structure. Vibrating rollers shall not be operated within 5 feet of any structure. Compaction by means of drop weights operating from a crane, hoist, or similar equipment is not permitted.

When compaction other than Class III compaction is specified, rockfill placed in trenches or other locations inaccessible to heavy equipment shall be compacted by manually controlled pneumatic or vibrating tampers or by equivalent methods approved by the engineer.

8. Compaction of bedding

Bedding shall be compacted according to the following requirements for the Class of compaction specified:

Class A compaction—Each layer of bedding shall be compacted to a relative density of not less than 70 percent as determined by ASTM Method D 4254.

Class I compaction—Each layer of bedding shall be compacted by at least two passes over the entire surface with a steel-drum vibrating roller weighing at least 5 tons and exerting a vertical vibrating force not less than 20,000 pounds at a frequency not less than 1,200 times per minute, or an approved equivalent method.

Class II compaction—Each layer of bedding shall be compacted by one of the following methods or by an equivalent method approved by the engineer:

- a. At least two passes over the entire surface with pneumatic rubber-tired roller exerting a minimum pressure of 75 pounds per square inch. A pass is defined as at least one passage of the roller wheel, track, tire, or drum over the entire surface of the bedding layer.
- b. At least four passes over the entire surface with the track of a crawler-type tractor weighing a minimum of 20 tons.
- c. Controlled movement of the hauling equipment so that the entire surface is traversed by a minimum of one tread track of the loaded equipment.

Class III compaction—No compaction is required beyond that resulting from the placing and spreading operations.

Heavy equipment shall not be operated within 2 feet of any structure. Vibrating rollers shall not be operated within 5 feet of any structure. Compaction by means of drop weights operating from a crane, hoist, or similar equipment is not permitted.

When compaction other than Class III is specified, bedding placed in trenches or other locations inaccessible to heavy equipment shall be compacted by manually controlled pneumatic or vibrating tampers or by equivalent methods approved by the engineer.

9. Measurement and payment

For items of work for which specific unit prices are established in the contract, the volume of each type of rockfill, including bedding, with the zone boundaries and limits specified on the drawings or established by the engineer is measured and computed to the nearest cubic yard by the method of average cross-sectional end areas.

Payment for each type of rockfill is made at the contract unit price for that type of fill. Except as otherwise specified in section 10 of this specification, such payment will constitute full compensation for all labor, equipment, material, and all other items necessary and incidental to the performance of the work including furnishing, placing, and compacting the bedding material.

Compensation for any type of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 10 of this specification.

10. Items of work and construction details

10. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Bid Item 5, Rockfill.

- (1) This item shall consist of furnishing and placing the rockfill at the locations and to the lines and grades shown on the construction drawings and/or as staked in the field by the COTR.
- (2) The rockfill shall be crushed limestone meeting the requirements of coarse aggregate, Size #57 and/or Size #1 as indicated on the drawings, in accordance with ASTM C 33. Placement shall be as shown in the drawings.
- (3) No rockfill shall be placed until the COTR has inspected and approved the foundation.
- (4) Placement shall be by Method 1 as described in Section 5 of this specification.
- (5) Compaction shall be Class III as described in Section 7 of this specification.
- (6) Section 9, "Measurement and Payment", of this specification is deleted in its entirety and is replaced as follows:

Payment shall be made at the lump sum price in the contract and such payment shall be considered as full compensation for all material, labor, equipment, tools and other items necessary and incidental to the completion of the work as described in this contract.

Such payment will constitute full compensation for related Subsidiary Item, "Removal of Water".

Construction Specification 45—Plastic Pipe

1. Scope

The work consists of furnishing and installing plastic pipe (except corrugated polyethylene tubing) and the necessary fittings and appurtenances as shown on the drawings or as specified herein.

2. Material

Pipe, fittings, and gaskets shall conform to the requirements of Material Specification 547, Plastic Pipe, and as specified in section 14 of this specification or as shown on the drawings.

Perforated pipe shall conform to the requirements of Material Specification 547, Plastic Pipe, and as specified in section 14 of this specification or as shown on the drawings.

Unless otherwise specified, concrete shall conform to the requirements of Construction Specification 32, Structure Concrete, and section 8 of this specification.

Unless otherwise specified, earth backfill shall conform to the requirements of Construction Specification 23, Earthfill.

Unless otherwise specified, drainfill shall conform to the requirements of Construction Specification 24, Drainfill.

3. Handling and storage

Pipe shall be delivered to the job site and handled by means that provide adequate support to the pipe and do not subject it to undue stresses or damage. When handling and placing plastic pipe, care shall be taken to prevent impact blows, abrasion damage, and gouging or cutting (by metal edges and/or surface or rocks). The manufacturer's special handling requirements shall be strictly observed. Special care shall be taken to avoid impact when the pipe must be handled at a temperature of 40 degrees Fahrenheit or less.

Pipe shall be stored on a relatively flat surface so that the barrels are evenly supported. Unless the pipe is specifically manufactured to withstand exposure to ultraviolet radiation, it shall be covered with an opaque material when stored outdoors for 15 days or longer.

4. Excavation

Excavation shall be in accordance with Construction Specification 21, Excavation, and section 14 of this specification or as shown on the drawings.

The pipe foundation shall be excavated a minimum of 4 inches lower than the pipe grade shown on the drawings or staked in the field whenever bedrock, boulders, cobbles, or other material that may cause pipe damage is encountered at planned pipe grade.

5. Laying the pipe

Plastic pipe conduits complete with fittings and other related appurtenances shall be installed to the lines and grades shown on the drawings or specified in section 14 of this specification. The pipe shall be installed so that there is no reversal of grade between joints unless otherwise shown on the drawings. The pipe shall not be dropped or dumped on the bedding or into the pipe trench. The ground surface near the pipe trench shall be free of loose rocks and stones greater than 1 inch in diameter. This ensures that rock will not be displaced and impact the pipe.

Just before placement, each pipe section shall be inspected to ensure that all foreign material is removed from inside the pipe. The pipe ends and the couplings shall be free of foreign material when assembled. At the completion of a work shift, all open ends of the pipeline shall be temporarily closed off using a suitable cover or plug.

Care shall be taken to prevent distortion and damage during hot or cold weather. During unusually hot weather (daytime high temperature of more than 90 °F), the pipe assembled in the trench shall be lightly backfilled or shaded to keep it as near to ground temperature as possible until final backfill is placed. Backfill operations should be performed during daily construction periods when the ground temperature and the temperature of the pipe do not vary more than 40 degrees Fahrenheit.

Perforated pipe shall be installed with the perforations down and oriented symmetrically about the vertical centerline. Perforations shall be clear of any obstructions on the inside and outside of the pipe when the pipe is approved by the engineer for backfill.

During installation, the pipe shall be firmly and uniformly bedded throughout its entire length, to the depth and in the manner specified in section 14 of this specification or as shown on the drawings. Bell-holes shall be placed in bedding material under bells, couplings, and other fittings to assure the pipe is uniformly supported throughout its entire length. Blocking or mounding beneath the pipe to bring the pipe to final grade is not permitted.

6. Pipe embedment

Earth bedding— The pipe shall be firmly and uniformly placed on compacted earthfill bedding or an in-place earth material bedding of ample bearing strength to support the pipe without noticeable settlement. The earth material on which the pipe is placed shall be of uniform density to prevent differential settlement.

Unless otherwise specified, a groove that closely conforms to the outside surface of the pipe shall be formed in the bedding. The depth of the groove shall be equal to or greater than 0.3 of the pipe diameter.

Earth bedding shall be compacted to a density not less than adjacent undisturbed in-place earth material or be compacted earth backfill. Earthfill material used for compacted earth bedding shall be free of rocks or stones greater than 1 inch in diameter and earth clods greater than 2 inches in diameter. The pipe shall be loaded sufficiently during the compaction of bedding under the haunches and around the sides of the pipe to prevent displacement from its final approved placement.

Sand, gravel, or crushed rock bedding— When sand, gravel, or crushed rock bedding is specified, the pipe shall be firmly and uniformly placed on the bedding material. Material for bedding shall not exceed 1 inch in diameter. Unless otherwise specified in section 14 of this specification or shown on the drawings, the coarse-grained bedding material shall be carefully placed and compacted to a depth equal to or greater than 0.3 of the diameter of the pipe above the bottom of the pipe. The pipe shall be loaded sufficiently during backfilling and compaction around the sides to prevent displacement of the pipe from its final approved placement.

Pipe encased in drainfill— The pipe shall be firmly and uniformly placed on bedding of specified drainfill. Drainfill shall be placed and compacted as specified in section 14 of this specification or as shown on the drawings to form a continuous uniform support around the entire circumference of the pipe. The pipe shall be loaded sufficiently during backfilling around the sides and during compaction to prevent displacement of the pipe.

7. Backfill

Initial backfill— Unless otherwise specified, initial backfill to 6 inches above the top of the conduit is required. Earth haunching and initial backfill material shall consist of soil material that is free of rocks, stones, or hard clods more than 1 inch in diameter. Coarse backfill material shall be the specified sand, gravel, crushed rock, or drainfill material.

Initial backfill shall be placed in two stages. In the first stage (haunching), backfill is placed to the pipe spring line (center of pipe). In the second stage, it is placed to 6 inches above the top of the pipe.

The first stage material shall be worked carefully under the haunches of the pipe to provide continuous support throughout the entire pipe length. The haunching backfill material shall be placed in layers that have a maximum thickness of about 6 inches and are compacted as specified in section 14 of this specification or as shown on the drawings. During compaction operations, care shall be taken to ensure that the tamping or vibratory equipment does not come in contact with the pipe and the pipe is not deformed or displaced.

When pressure testing is not specified, the pipe shall be covered with a minimum of 6 inches of backfill material as soon as possible following assembling of the pipe in the trench, but not later than within the same day that placement has occurred. When pressure testing is specified, sufficient backfill material shall be placed over the pipe to anchor the conduit against movement during pressure testing activities.

Final backfill— Final backfill shall consist of placing the remaining material required to complete the backfill from the top of the initial backfill to the ground surface, including mounding at the top of the trench. Final backfill material within 2 feet of the top of the pipe shall be free of debris or rocks larger than 3 inches nominal diameter. Coarse backfill material shall be the specified sand, gravel, crushed rock, or drainfill. Final backfill shall be placed in approximately uniform, compacted layers. Final backfill compaction requirements shall be as specified in section 14 of this specification or as shown on the drawings.

Vehicles or construction equipment shall not be allowed to cross the pipe until the minimum earth cover and required density as specified in section 14 of this specification has been obtained.

8. Pipe encasement in concrete

Concrete encasement shall be carefully placed to form a continuous uniform support around the entire circumference of the pipe as specified in section 14 of this specification or as shown on the drawings. Pipes encased in concrete shall be securely anchored to prevent movement of the pipe during concrete placement. A clear distance of 1.5 inch shall be maintained between the pipe and the reinforcing steel.

The concrete for the encasement shall conform to the requirements of Construction Specification 32, Structure Concrete, for Class 3000M concrete unless otherwise specified.

9. Joints

Unless otherwise specified in section 14 of this specification or shown on the drawings, joints shall be either bell and spigot type with elastomeric gaskets, coupling type, solvent cement bell and spigot, or jointed by butt heat fusion. When a lubricant is required to facilitate joint assembly, it shall be a type having no deleterious affect on the gasket or pipe material.

Pipe joints shall be watertight at the pressures specified except where unsealed joints are indicated.

Pipe shall be installed and joined in accordance with the manufacturer's recommendations. Laying deflections and joint fitting or stab depths shall be within the manufacturer's recommended tolerances.

When solvent cement joints are specified for PVC or ABS pipe and fittings, they shall be made in accordance with the following ASTMs and the related appendix of each ASTM; D 2855 for PVC pipe and fittings and D 2235 for ABS pipe and fittings.

Flanged, banded, heat-fusion, or elastomeric-sealed mechanical joints shall be used when joining polyethylene (PE) and high density polyethylene (HDPE) pipe and fittings unless otherwise specified in section 14 of this specification or as shown on the drawings.

Pipe ends shall be cut square and be deburred to provide a uniform, smooth surface for the jointing process. Reference marks shall be placed on the spigot ends to assist in determining when proper seating depth has been achieved within the joint.

10. Fittings

Unless otherwise specified, steel fittings, valves, and bolted connections shall be painted or coated as recommended by the manufacturer.

Fittings for nonpressure pipe shall be of the same or similar material as the pipe and shall provide the same durability, watertightness, and strength as the pipe unless otherwise specified.

11. Thrust blocks and anchors

When specified, concrete thrust blocks and anchors shall be installed as shown on the drawings or specified in section 14 of this specification.

The concrete for the thrust blocks and anchors shall conform to the requirements of Construction Specification 32, Structure Concrete, for Class 3000M concrete unless otherwise specified in section 14 of this specification.

The thrust block cavity shall be hand dug into undisturbed soil or previously placed compacted backfill. The cavity shall be formed with soil or wood to hold the freshly placed concrete without displacement until an initial set has occurred.

When excavation beyond the designated trench widths and depths as shown on the drawings or specified in section 14 of this specification occurs at locations where installation of concrete thrust blocks is required, the contractor shall install an alternative thrust block provision. The concrete thrust block shall have a thickness of one pipe diameter and a contact face area that shall be formed against the pipe as shown on the drawings or specified in section 14 of this specification. Backfill shall be placed on all sides of the thrust block and to the sides of the excavation. It shall be compacted as specified for initial backfill.

12. Pressure testing

Method 1—Pressure testing of the completed conduit is not required.

Method 2—The conduit shall be tested for leaks in the following manner:

a. Before pressure testing:

- (1) Joints of the assembled pipeline shall be allowed to cure as recommended by the manufacturer.
- (2) Pipeline shall be flushed and cleaned.
- (3) All concrete anchors and thrust blocks shall be in place and allowed to cure for a minimum of 3 days.
- (4) Earth backfill shall be sufficient to anchor the conduit against movement during the pressure testing and shall be compacted as specified in Section 14 of this specification or as shown on the drawings.

- (5) The conduit shall be braced, anchored, or both, at each end to restrict all potential pipe movement.
- (6) The ends of the conduit shall be plugged. The upstream plug shall have a standpipe installed vertically having a minimum diameter of 2 inches and shall be equipped with a shutoff valve. All high points in the line shall be vented to evacuate air pockets. The conduit and the standpipe shall be slowly filled with water such that no air is entrapped during the filling operation. After filling is complete, all vents shall be closed.
- b. During pressure testing, the water level in the standpipe shall be continuously maintained at a minimum of 10 feet above the highest invert elevation of the conduit for no less than 2 hours unless otherwise specified in section 14 of this specification or as shown on the drawings.

The volume of water leakage in the 2-hour test period shall be recorded. The maximum allowable leakage (L) in gallons per hour shall not exceed 0.02 times the nominal pipe diameter (D) in inches for each 1,000 feet of pipe line, which is about 50 pipe joints ($L = 0.02 \times D$).

- c. When observed leakage exceeds the allowable, leaks shall be sealed by replacement of pipe and fittings as necessary. The conduit shall be retested as described above. This procedure shall be repeated until the conduit leakage does not exceed the allowable specified above.

The contractor shall be fully responsible for any and all work required to correct leakage exceeding the amount specified.

Method 3—The conduit shall be tested for leaks in the following manner:

- a. Before pressure testing:
 - (1) Joints of the assembled pipeline shall be allowed to cure as recommended by the manufacturer.
 - (2) Pipeline shall be flushed and cleaned.
 - (3) All concrete anchor and thrust blocks shall be in place and allowed to cure for at least 3 days.
 - (4) Earth backfill shall be sufficient to anchor the conduit against movement during the pressure testing and compacted as specified in section 14 of this specification or as shown on the drawings.
 - (5) The conduit shall be braced and/or anchored at each end to prevent all potential pipe movement.
 - (6) The ends of the conduit shall be plugged, and a pressure gauge shall be attached to the upstream and downstream ends. All high points along the pipeline shall be vented to permit the complete removal of all air within the pipeline. The conduit shall be slowly filled with water such that no air is entrapped during the filling operations.
- b. The testing pressure specified in section 14 of this specification shall be continuously maintained at the upstream gauge for a minimum of 2 hours. The pressure at the downstream gauge shall not exceed the pressure rating of the pipe.
- c. The volume of water leakage for the 2-hour test period shall be recorded. Maximum allowable leakage shall be in accordance with the following:

Allowable leakage for plastic pipe
(gal/hr/1,000 feet, or 50 pipe joints) ^{1/}

Nominal pipe size (in)	Test pressure in the pipeline (lb/in ²)			
	50	100	150	200
	----- Allowable leakage -----			
4	.19	.27	.33	.38
6	.29	.41	.50	.57
8	.38	.54	.66	.76
10	.48	.68	.83	.96
12	.57	.81	.99	1.15
14	.67	.95	1.16	1.34
15	.72	1.02	1.25	1.44
16	.76	1.07	1.32	1.52
18	.86	1.22	1.49	1.72

1/ Computation basis
$$L = \frac{ND\sqrt{P}}{7,400}$$

where:

- L = allowable leakage in gallons per hour
- N = number of joints (pipe and fittings)
- D = nominal diameter of pipe in inches
- P = test pressure in pounds per square inch

- d. When observed leakage exceeds the allowable, leaks shall be sealed by replacement of pipe and fittings as necessary. The conduit shall be retested as described in this section. The procedure shall be repeated until the conduit leakage does not exceed the allowable specified above.

The contractor shall be fully responsible for any and all work required to correct leakage exceeding the amount specified.

13. Measurement and payment

Method 1—For items of work for which specific unit prices are established in the contract, the quantity of each kind, size, and class of pipe is determined to the nearest foot by measurement of the laid length along the crown centerline of the conduit. Payment for each kind, size, and class of pipe is made at the contract unit price for that kind, size, and class. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe including excavation, shoring, backfill, bedding, thrust blocks, and all fittings, appurtenances, and other items necessary and incidental to the completion of the work. Payment for appurtenances listed separately in the bid schedule is made at the contract prices for those items.

Method 2—For items of work for which specific unit prices are established in the contract, the quantity of each kind, size, and class of pipe is determined as the sum of the nominal laying lengths of the sections used. Payment for each kind, size, and class of pipe is made at the contract unit price for the kind, size, and class. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe including excavation, shoring, backfill, bedding, thrust blocks, and all fittings, appurtenances, and other items necessary and incidental to the completion of the work. Payment for appurtenances listed separately in the bid schedule is made at the contract prices for those items.

Method 3—For items of work for which specific unit prices are established in the contract, the quantity of each kind, size, and class of pipe is determined to the nearest foot by measurement of the laid length along the crown centerline of the conduit. Payment for each kind, size, and class of pipe is made at the contract unit price for the kind, size, and class. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe including shoring, all fittings, thrust blocks, appurtenances, and other items necessary and incidental to the completion of the work. Payment for appurtenances listed separately in the bid schedule is made at the contract prices for those items.

Method 4—For items of work for which specific unit prices are established in the contract, the quantity of each kind, size, and class of pipe is determined as the sum of the nominal laying lengths of the pipe sections used. Payment for each kind, size, and class of pipe is made at the contract unit price for that kind, size, and class. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe including shoring, all fittings, thrust blocks, appurtenances, and other items necessary and incidental to the completion of the work. Payment for appurtenances listed separately in the bid schedule is made at the contract prices for those items.

Methods 3 and 4—Excavation, backfill, and bedding is paid separately under their respective bid items.

All measurement and payment methods—Compensation for any items of work described in the contract, but not listed in the bid schedule, is included in the payment for the item of work to which it is made subsidiary. Such items and items to which they are made subsidiary are identified in section 14 of this specification.

14. Items of work and construction details

14. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Subsidiary Item , Plastic Pipe

- 1) This item shall consist of furnishing and installing the plastic pipe and necessary fittings to extend the existing plastic pipe at the locations shown in the "Pipe Schedule" table in the drawings and as shown on the drawings. Final location will be determined in the field by the COTR.
- 2) It shall be the contractor's responsibility to field verify the locations, numbers, pipe sizes (diameter, SDR, length), material types and coupler or banding types prior to procuring the pipe, fittings and pipe support materials for use in this contract.
- 3) Any additional plastic pipes and fittings needed to complete the rerouting shall be Poly Vinyl Chloride. The SDR shall be the same as existing pipe and shall meet Material Specification 547.
- 4) Joints per Section 9 shall be solvent cement bell and spigot.
- 5) Pressure testing as per Section 12 shall not be necessary and Method 1 will apply.
- 6) Plastic straight pipe repair shall be carried on concurrently with the other slope repair work along the channel.
- 7) Section 13, "Measurement and Payment", of this specification is deleted in its entirety and is replaced as follows:

For items of work for which specific lump sum prices are established in the contract, payment for plastic pipe structures is made at the contract lump sum price. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe including shoring, all fittings, thrust blocks, appurtenances, and other items necessary and incidental to the completion of the work, which includes, except as otherwise specified, required excavation, dewatering, and earth backfill.

Such payment will constitute full compensation for related Subsidiary Item, Timber Fabrication, Timber Pipe Support.

Construction Specification 51—Corrugated Metal Pipe

1. Scope

The work consists of furnishing and placing circular, arched, or elliptical corrugated metal pipe and the necessary fittings.

2. Material

Pipe and fittings shall conform to the requirements of Material Specification 551, Coated Corrugated Steel Pipe, or Material Specification 552, Aluminum Corrugated Pipe, whichever is specified.

Unless otherwise specified in section 11 of this specification, perforated pipe furnished shall conform to the requirements for Class I perforations as described in ASTM A 760 or A 762.

3. Coupling bands and hardware

Pipe joint coupling bands shall be provided meeting the requirements specified in section 11 of this specification.

Hardware consisting of coupling bands and band fastening devices, such as connecting bolts, rods, lugs, and angles used in conjunction with zinc-coated iron or steel pipe, shall be galvanized by the hot-dip method. Hardware used in conjunction with aluminum pipe and aluminum or aluminum-zinc alloy-coated iron and steel pipe shall be of the same material as the pipe except that hot-dip galvanized or cadmium-plated fasteners may be used. The surface of all band-fastening devices for pipe specified with bituminous or polymer coating shall be coated with asphalt-mastic material meeting the requirements of ASTM A 849. The coupling band shall be coated similar to that specified for the pipe unless otherwise specified in section 11 of this specification.

Coupling bands shall be installed to provide straight alignment of the connecting pipe ends. Unless otherwise specified in section 11 of this specification, the bandwidth shall be as specified in ASTM A 760 and A 762. The bands shall be positioned to overlap adjacent pipe ends equally. The coupling bands shall be corrugated to match the corrugations of the pipe section ends being connected.

4. Fabrication

Fabrication of appurtenant sections shall be performed as shown on the drawings and described in section 11 of this specification. The items may consist of inlet sections, outlet sections, end sections, elbows, skew or beveled sections, rod reinforced ends, cut-off collars, or headwalls. Fabrication of these appurtenant sections shall be made from metallic-coated material identical to that from which the attached pipe is fabricated. Fabrication shall be of a quality and finished workmanship equal to that required for the pipe.

5. Handling the pipe

The contractor shall furnish equipment as necessary to install the pipe without damaging the pipe or coating. The pipe shall be transported and handled in a manner to prevent damage to the pipe and coating.

6. Laying and bedding the pipe

Unless otherwise specified, the pipe shall be installed in accordance with the manufacturer's recommendations. Pipe shall be installed so no reversal of grade between joints results unless otherwise shown on the drawings. The pipe shall be installed with the outside laps of circumferential joints pointing upstream and with longitudinal laps at the sides near the vertical mid-height of the pipe.

Field welding of corrugated galvanized iron or steel pipe is not permitted. The pipe sections shall be joined with fabricator-supplied coupling bands meeting the specified joint requirements. The coupling shall be installed as recommended by the fabricator.

The pipe shall be firmly and uniformly bedded throughout its full length to the depth and in the manner specified on the drawings.

Perforated pipe shall be installed with the perforations down and oriented symmetrically about a vertical centerline. Perforations shall be clear of any obstructions at the time the pipe is installed in its final position.

The pipe shall be loaded sufficiently during backfilling to prevent displacement from line and grade and to maintain full contact with the bedding during the placement operations.

7. Strutting

When required, struts or horizontal ties shall be installed in the manner specified on the drawings. Struts and ties shall remain in position until the backfill has been placed above the top of the pipe to a height of 5 feet or the pipe diameter, whichever is greater, or to the surface of the completed earth backfill when the fill height is less than 5 feet above the top of the pipe. The contractor shall remove the struts or ties following completion of the earth backfill requirements that apply.

8. Embedment in concrete

Special treatment shall be provided to the pipe surface when embedded or attached to concrete and the pipe material is aluminum or aluminum-coated and aluminum-zinc alloy-coated. Potential contact surfaces in contact with concrete and masonry surfaces shall be coated with two coats of a bituminous paint of the cutback type. Placement of the pipe shall be such that direct metal-to-metal contact with other metallic material, such as embedded steel reinforcement or water control gates, is prevented.

9. Repair of damaged coating

Any damage to the metallic coating shall be repaired by cleaning the damaged surface area by sand blasting, power disk sanding, or wire brushing. All loose and cracked coating, dirt, and any products of corrosion shall be removed before application of paint. Oil and grease material shall be removed by use of a solvent. The surface shall be clean and dry during the painting period and until the coating has completely dried.

Painting shall be accomplished by one of the following options based upon installed exposure conditions of the pipe as determined by the engineer.

Normal exterior or interior atmospheric exposure:

- a. Zinc dust - zinc oxide primer, ASTM D 79 and D 520
- b. Single package, moisture cured urethane prime in silver metallic color, or
- c. Zinc-rich cold galvanized compound, brush, or aerosol application

Submergence in water exposure:

- a. Zinc dust - zinc oxide primer, ASTM D 79 and D 520
- b. Zinc dust paint, ASTM D 4146

When the metallic coating is damaged in any individual area larger than 12 square inches or if more than 0.2 percent of the total surface area of a single pipe section is damaged, that section of pipe will be rejected.

Breaks or scuffs in bituminous coatings that are less than 36 square inches in area shall be repaired by applying two coats of hot-asphaltic paint or a coating of cold-applied bituminous mastic. The repair coating shall be a minimum of 0.05 inch thick after hardening and shall bond securely and permanently to the pipe and coating. The material shall meet the minimum physical requirements for bituminous coating in ASTM A 849 and A 885. Whenever individual breaks exceed 36 square inches in area or when the total area of breaks exceeds 0.5 percent of the total surface area of an individual pipe section, that section of pipe will be rejected.

Bituminous coating damaged by welding of coated pipe or pipefittings shall be repaired as specified in this section for breaks or scuffs in bituminous coatings.

Breaks or scuffs in polymer coatings that are less than 36 square inches in area shall be repaired by the application of a polymer material similar to and compatible with the durability, adhesion, and appearance of the original polymer coating, as described in ASTM A 849, paragraph 6.8. The repair coating shall be a minimum thickness of 0.010 inch (10 mils) after drying. Whenever individual breaks exceed 36 square inches in area or when the total area of breaks exceeds 0.5 percent of the total surface area of the individual pipe section, that section of pipe will be rejected.

10. Measurement and payment

Method 1—For items of work for which specific unit prices are established in the contract, the quantity of each type, class, size, and gauge of pipe is determined to the nearest 0.1 foot by measurement of the laid length of the pipe along the centerline of the pipe. Payment for each type, class, size, and gauge of pipe is made at the contract unit price for that type, class, size and gauge of pipe. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe and fittings and all other items necessary and incidental to the completion of the work except items designated as *special fittings*. Special fittings are those sections of pipe requiring special fabrication to meet layout requirements. Payment for special fittings is made at the contract unit price for special fittings (CMP).

Method 2—For items of work for which specific unit prices are established in the contract, the quantity of each type, class, size, and gauge of pipe is determined as the sum of the nominal laying lengths of the pipe sections installed. Payment for each type, class, size, and gauge of pipe is made at the contract unit price for that type, class, size, and gauge of pipe. Such payment constitutes full compensation for furnishing, transporting, and installing the pipe and fittings and all other items necessary and incidental to the completion of the work except items designated as *special fittings*. Special fittings are those sections of pipe requiring special fabrication to meet layout requirements. Payment for special fittings is made at the contract unit price for special fittings (CMP).

Method 3—For items of work for which specific lump sum prices are established in the contract, payment for corrugated metal pipe structures is made at the contract lump sum price. Such payment constitutes full compensation for furnishing, fabricating, transporting, and installing the pipe structure complete with metal pipe, fittings, and appurtenances, and all other items necessary and incidental to completion of the work, which includes, except as otherwise specified, required excavation, dewatering, and earth backfill.

All Methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule, is included in the payment for the item of work to which it is made subsidiary. Such items and items to which they are made subsidiary are identified in section 11 of this specification.

11. Items of work and construction details

11. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefor are:

a. Bid Item 8, Corrugated Metal Pipe

- (1) This item shall consist of furnishing and installing the corrugated metal pipe as shown on the "Pipe Schedule" table in the drawings at the locations and in conformance with the specifications. Final location will be determined in the field by the COTR.
- (2) It shall be the contractor's responsibility to field verify the locations, numbers and pipe sizes (diameter, gage, length), material types and coupler or banding types prior to procuring the pipe, fittings and pipe support materials for use in this contract.
- (3) The pipe gages shall be as specified in the table below:

Pipe Diameter	Pipe Gage
Less than 18"	14
Greater than or equal to 18"	12

- (4) All Corrugated Metal Pipe shall conform to Material Specification 551. No pipe will be accepted until the sheet manufacturers' metallurgical test report and suppliers' bill of lading, showing respective heat numbers, have been approved by the Contracting Officer.
- (5) The corrugated metal pipe shall be installed to the lines and grades as shown on the drawings and staked in the field.
- (6) Each corrugated metal pipe listed in the "Pipe Schedule" table in the drawings shall be supplied in one continuous joint at the length specified. In the event that one continuous joint cannot be shipped, multiple sections of pipe will be permitted. Only one (1) coupling per 20 feet of pipe length shall be allowed.
- (7) The corrugated metal pipe shall be Type I. It shall be fabricated with helical corrugations and have continuous lock seams that are welded a minimum of 2 inches at each end of the pipe section.
- (8) All corrugated metal pipe shall be polymer coated. The polymer coating shall be Grade 10/10. The pipe ends shall be re-rolled to annular corrugations for a minimum length of 12 inches and shall be neatly trimmed and finished in a workman-like manner. Exposed ends shall be treated with zinc dust-zinc oxide primer, conforming to Material Specification 551. The ends shall then be painted with polymer paint in accordance with the recommendations of the manufacturer. The polymer paint shall be of a type compatible with the polymer coating of the pipe. All welds and exposed metal shall be thoroughly cleaned and painted with two coats of zinc dust-zinc oxide primer, conforming to Material Specification 551.
- (9) Polymer coating damaged by welding shall be painted with polymer paint in accordance with the recommendations of the manufacturer. The polymer paint shall be of a type compatible with the polymer coating of the pipe and installed in a uniform thickness. All repaired areas of

polymer coating shall have a minimum thickness of ten (10) mils. The rerolled ends and lock seams shall be painted inside and outside with polymer paint as specified above except the inside lock seams of pipe smaller than thirty (30) inches diameter do not have to be painted.

- (10) At all times the pipe shall be handled with equipment designed to prevent damage to the pipe or coatings. Bare cables, chains, hooks, metal bars or narrow skids shall not be permitted to come in contact with the pipe.
- (11) The COTR shall inspect the pipe at the placement location, and if the pipe is found damaged, the damaged area will be repaired to a pre-damaged condition at no cost to the contracting agency. If the polymer coating is damaged in any individual area larger than 12 square inches or if more than 0.2 percent of the total surface area of a length of pipe is damaged, the length will be rejected.
- (12) Joint compound or closed cell expanded rubber gaskets shall be used with each connecting band. The joint compound shall be bitumastic "50" or equal. Closed cell rubber gaskets shall be twelve (12) inches wide, three-eighths (3/8) inch thick, unstretched diameter ten (10) percent less than nominal pipe size and shall comply with ASTM Specification D 1056-85, Grade 2C2.
- (13) Coupling bands shall be used as specified below. The contractor shall furnish any additional joint compound and/or additional rods and tank lugs he/she deems necessary to produce a watertight joint at his/her expense. The contractor is responsible for determining existing pipe corrugations.
 - a) The six (6) rod connecting band shall be used when joining the new pipe to existing CMP with annular corrugations at the proposed joint.
 - b) Flat couplers, minimum 2' wide, shall be used when joining the new pipe to existing CMP with helical corrugations at the proposed joint.
- (14) Existing pipe shall be cut and the section replaced if the end of the existing pipe is not suitable for creating a smooth and watertight connection to the new pipe.
- (15) Measurement and Payment shall be by Method 3. Such payment will constitute full compensation for related Subsidiary Item, Timber Fabrication, Timber Pipe Support.

Construction Specification 83—Timber Fabrication and Installation

1. Scope

The work shall consist of the construction of timber structures and timber parts of composite structures.

2. Material

Structural timber and lumber shall conform to the requirements of Material Specification 584. Treated timber and lumber shall be impregnated with the specified type and quantity of preservative and in the manner specified in Material Specification 585.

Hardware, except cast iron, shall be galvanized as specified for iron and steel hardware in Material Specification 582. Unless otherwise specified, structural steel shapes, plates, and rods shall not be galvanized. Nuts, driftbolts, dowels, and screws shall be either wrought iron or steel.

Steel bolts shall conform to the requirements of ASTM A 307. When galvanized or zinc-coated bolts are specified, the zinc coating shall conform to the requirements of Material Specification 582.

Washers shall be ogee gray iron castings or malleable iron castings unless washers cut from medium steel or wrought iron plate are specified on the drawings or in section 7 of this specification. Cast washers shall have a thickness equal to the diameter of the bolt and a diameter equal to four times the thickness. The thickness for plate washers shall be equal to half the diameter of the bolt, and the sides of the square shall be equal to four times the diameter of the bolt. Holes in washers shall have a maximum diameter of 1/8 inch larger than the diameter of the bolt. Split ring connectors, tooth ring connectors, and pressed steel shear plate connectors shall be manufactured from hot-rolled, low carbon steel conforming to the requirements of ASTM A 711, Grade 1015. Malleable iron shear plate connectors and spike grid connectors shall be manufactured in conformance with the requirements of ASTM A 47, Grade No. 35018. All connectors shall be of approved design and the type and size specified.

Structural shapes, rods, and plates shall be structural steel conforming to the requirements of Material Specification 581. No welds are permitted in truss rods or other main members of trusses or girders.

3. Workmanship

All framing shall be true and exact. Timber and lumber shall be accurately cut and assembled to a close fit and shall have even bearing over the entire contact surface. No open or shimmed joints will be accepted. Nails and spikes shall be driven with just sufficient force to set the heads flush with the surface of the wood. Deep hammer marks in wood surfaces shall be considered evidence of poor workmanship and may be sufficient cause for rejection of the work.

Holes for round driftpins and dowels shall be bored with a bit 1/16 inch smaller in diameter than that of the driftpin or dowel to be installed. The diameter of holes for square driftpins or dowels shall be equal to one side of the driftpin or dowel. Holes for lag screws shall be bored with a bit not larger than the body of the screw at the base of the thread.

Washers shall be used in contact with all bolt heads and nuts that would otherwise be in contact with wood. Cast iron washers shall be used when the bolt will be in contact with earth. All nuts shall be checked or burred effectively with a pointed tool after finally tightened.

Unless otherwise specified, surfacing, cutting, and boring of timber and lumber shall be completed before treatment. If field cutting or field repair of treated timber and lumber is approved, all cuts and abrasions shall be carefully trimmed and coated with two paint-on or swab-applied applications of a wood

preservative that is not less than 5 percent (by weight) pentachlorophenol. A copper metal solution of 2 percent (by weight) copper naphthenate may be used as a replacement for pentachlorophenol, which is a controlled substance. After timber assembly, any unfilled holes shall be plugged with tightly fitting wooden plugs that have been treated with preservative as specified.

4. Handling and storing material

All timber and lumber stored at the site of the work shall be neatly stacked on supports a minimum of 12 inches above the ground surface and protected from the weather by suitable covering(s). Untreated material shall be staked and stripped to permit free circulation of air between the tiers and courses. Treated timber may be close-staked. The ground surface for the stockpile of timber and lumber shall be free of weeds and rubbish. The use of cant hooks, peavies, or other pointed tools except end hooks is not permitted in the handling of structural timber and/or lumber. Treated timber shall be handled with rope slings or by other methods that prevent the breaking or bruising of outer fibers or penetration of the surface in any manner.

5. Painting

Except as otherwise specified, surfaces designated for painting shall be prepared and painted in accordance with Construction Specification 84.

6. Measurement and payment

Method 1—The unit of measurement of lumber and timber is the number of thousand feet board measure (MBM) of each type, size, species, and grade of lumber and timber installed in the completed structure. The quantity of each type, size, species, and grade is computed from the nominal dimensions and actual lengths of the pieces in the completed structure and does not include waste timber used for erection purposes (such as falsework or temporary sheeting and bracing) or any part of any pile or other round timber. The total quantity of lumber and timber in each type, size, species, and grade is computed to the nearest 0.01 MBM.

The unit of measurement of plywood is the number of square feet of each type, species, grade, and thickness installed in the completed structure.

Payment for each type, size, species, and grade of lumber and timber is made at the contract unit price for that type, size, species, and grade. Payment for each type, species, grade, and thickness of plywood is made at the contract unit price for that type, species, grade, and thickness. Such payment is considered full compensation for completion of the work.

Method 2—No measurement of material quantities is made. Payment for each structure, complete in place, is made at the contract lump sum price for that structure. Such payment is considered full compensation for completion of the work.

Method 3—For items of work for which specific unit prices are established in the contract, measurement and payment for each structure unit except those for which a linear foot payment is established is counted and payment made at the contract unit price. Items for which a linear foot payment is established are measured to the nearest linear foot, and payment is made at the contract unit prices as appropriate. Such payment is considered full compensation for completion of the work.

All methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule, is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 7 of this specification.

7. Items of work and construction details

7. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefore are:

a. Subsidiary Item, Timber Fabrication, Timber Pipe Supports

- (1) This item shall consist of furnishing all timber piles, pipe supports and other related materials and labor necessary for the construction and installation of the pipe supports as shown on the drawings and/or as staked in the field.

Location	Approx. Sta.
Soniat Canal, Site #1	Sta. 0+37
Soniat Canal, Site #1	Sta. 2+19
Soniat Canal, Site #2	Sta. 0+57
Soniat Canal, Site #3	Sta. 0+74
Soniat Canal, Site #3	Sta. 0+98
Soniat Canal, Site #7	Sta. 1+80

- (2) All lumber and pipe supports shall be No 2 Southern Yellow Pine or Douglas Fir, meeting Material Specifications 512 and 584. Piles shall have the minimum diameter shown on the drawings. All lumber and pipe support piles shall be pressure treated to a minimum of 0.4 pounds per cubic foot net retention with chromated copper arsenate (CCA) conforming to Material Specification 585. Testing of lumber as directed by the Contracting Officer shall be at the contractor's expense.
- (3) All nails, bolts, washers and other metal hardware shall be as shown on the drawings and shall be galvanized and conform to Material Specifications 581 and 582.
- (4) No separate payment will be made for this item. Compensation for Subsidiary Item, "Timber Pipe Supports" will be included in the payment for Bid Item 8, Corrugated Metal Pipe and Bid Item 9, Plastic Pipe, to which it is associated.

Construction Specification 95—Geotextile

1. Scope

This work consists of furnishing all material, equipment, and labor necessary for the installation of geotextiles.

2. Quality

Geotextiles shall conform to the requirements of Material Specification 592 and this specification.

3. Storage

Before use, the geotextile shall be stored in a clean, dry location out of direct sunlight, not subject to extremes of either hot or cold temperatures, and with the manufacturer's protective cover undisturbed. Receiving, storage, and handling at the job site shall be in accordance with the requirements listed in ASTM D 4873.

4. Surface preparation

The surface on which the geotextile is to be placed shall be graded to the neat lines and grades as shown on the drawings. It shall be reasonably smooth and free of loose rock and clods, holes, depressions, projections, muddy conditions, and standing or flowing water (unless otherwise specified in section 7 of this specification).

5. Placement

Before the geotextile is placed, the soil surface will be reviewed for quality assurance of the design and construction. The geotextile shall be placed on the approved prepared surface at the locations and in accordance with the details shown on the drawings and specified in section 7 of this specification. It shall be unrolled along the placement area and loosely laid, without stretching, in such a manner that it conforms to the surface irregularities when material or gabions are placed on or against it. The geotextile may be folded and overlapped to permit proper placement in designated area(s).

Method 1—The geotextile shall be joined by machine sewing using thread material meeting the chemical requirements for the geotextile fibers or yarn. The sewn overlap shall be 6 inches, and the sewing shall consist of two parallel stitched rows at a spacing of about 1 inch and shall not cross (except for any required re-stitching). The stitching shall be a lock-type stitch. Each row of stitching shall be located a minimum of 2 inches from the geotextile edge. The seam type and sewing machine to be used shall produce a seam strength, in the specified geotextile, that provides a minimum of 90 percent of the tensile strength in the weakest principal direction of the geotextile being used, when tested in accordance with ASTM D 4884. The seams may be factory or field sewn.

The geotextile shall be temporarily secured during placement of overlying material to prevent slippage, folding, wrinkling, or other displacement of the geotextile. Unless otherwise specified, methods of securing shall not cause punctures, tears, or other openings to be formed in the geotextile.

Method 2—The geotextile shall be joined by overlapping a minimum of 18 inches (unless otherwise specified) and secured against the underlying foundation material. Securing pins, approved and provided by the geotextile manufacturer, shall be placed along the edge of the panel or roll material to adequately hold it in place during installation. Pins shall be steel or fiberglass formed as a U, L, or T shape or contain "ears" to prevent total penetration through the geotextile. Steel washers shall be provided on all but the U-shaped pins. The upstream or upslope geotextile shall overlap the abutting downslope geotextile. At vertical laps, securing pins shall be inserted through the bottom layers along a line through approximately the mid-point of the overlap. At horizontal laps and across slope labs, securing shall be inserted

through the bottom layer only. Securing pins shall be placed along a line about 2 inches in from the edge of the placed geotextile at intervals not to exceed 12 feet unless otherwise specified. Additional pins shall be installed as necessary and where appropriate to prevent any undue slippage or movement of the geotextile. The use of securing pins will be held to the minimum necessary. Pins are to remain in place unless otherwise specified.

Should the geotextile be torn or punctured, or the overlaps or sewn joint disturbed, as evidenced by visible geotextile damage, subgrade pumping, intrusion, or grade distortion, the backfill around the damaged or displaced area shall be removed and restored to the original approved condition. The repair shall consist of a patch of the same type of geotextile being used and overlaying the existing geotextile. When the geotextile seams are required to be sewn, the overlay patch shall extend a minimum of 1 foot beyond the edge of any damaged area and joined by sewing as required for the original geotextile except that the sewing shall be a minimum of 6 inches from the edge of the damaged geotextile. Geotextile panels joined by overlap shall have the patch extend a minimum of 2 feet from the edge of any damaged area.

Geotextile shall be placed in accordance with the following applicable specification according to the use indicated in section 7:

Slope protection— The geotextile shall not be placed until it can be anchored and protected with the specified covering within 48 hours or protected from exposure to ultraviolet light. In no case shall material be dropped on uncovered geotextile from a height of more than 3 feet.

Subsurface drains— The geotextile shall not be placed until drainfill or other material can be used to provide cover within the same working day. Drainfill material shall be placed in a manner that prevents damage to the geotextile. In no case shall material be dropped on uncovered geotextile from a height of more than 5 feet.

Road stabilization— The geotextile shall be unrolled in a direction parallel to the roadway centerline in a loose manner permitting conformation to the surface irregularities when the roadway fill material is placed on its surface. In no case shall material be dropped on uncovered geotextile from a height of more than 5 feet. Unless otherwise specified, the minimum overlap of geotextile panels joined without sewing shall be 24 inches. The geotextile may be temporarily secured with pins recommended or provided by the manufacturer, but they shall be removed before the permanent covering material is placed.

6. Measurement and payment

Method 1—For items of work for which specific unit prices are established in the contract, the quantity of geotextile for each type placed within the specified limits is determined to the nearest specified unit by measurements of the covered surfaces only, disregarding that required for anchorage, seams, and overlaps. Payment is made at the contract unit price. Such payment constitutes full compensation for the completion of the work.

Method 2—For items of work for which specific unit prices are established in the contract, the quantity of geotextile for each type placed with the specified limits is determined to the nearest specified unit by computing the area of the actual roll size or partial roll size installed. The computed area will include the amount required for overlap, seams, and anchorage as specified. Payment is made at the contract unit price. Such payment constitutes full compensation for the completion of the work.

Method 3—For items of work for which specific lump sum prices are established in the contract, the quantity of geotextile is not measured for payment. Payment for geotextiles is made at the contract lump sum price and constitutes full compensation for the completion of the work.

All methods—The following provisions apply to all methods of measurement and payment. Compensation for any item of work described in the contract, but not listed in the bid schedule, is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in section 7 of this specification.

7. Items of work and construction details

7. Items of work and construction details

Items of work to be performed in conformance with this specification and the construction details therefor are:

a. Bid Item 6, Geotextile

- (1) This item shall consist of furnishing and placing geotextile under the rockfill and/or rock riprap at the location and to the extent shown on the drawings.
- (2) The geotextile shall be a non-woven Class I geotextile meeting the requirements of Table 592-2 of Material Specification 592.
- (3) The foundation shall be inspected and approved by the COTR prior to placement of the geotextile.
- (4) Placement shall be by Method 2 with the following additions: “The pinning interval shall not exceed 6 feet and the securing pins shall be a minimum of 12 inches in length. Overlaps shall be made in the direction of the flow or in a manner to prevent the stream flow from getting under the geotextile. Geotextile placed in the wet (under normal water level conditions) may be secured by the placement of the rockfill and/or rock riprap. ***When the contractor selects to place the geotextile in the wet, he shall provide sufficient geotextile and a method of placement to assure complete coverage of the area specified in paragraph (1).*** Any excess geotextile shall be cut off and disposed of at an approved landfill”. The use of the Geotextile shall be for “*Slope protection*”.
- (5) Payment shall be by Method 3 as described in Section 6 of this specification.

Material Specification 302—Coconut Fiber Erosion Control Revegetation Mat

1. Scope

The specification covers the type and quality of erosion control revegetation mat.

2. Materials

The Erosion Control and Revegetation Mat (ECRM) shall consist of a consistent thickness with the coconut fiber evenly distributed over the entire area of the mat. The coconut fibers shall be covered on the top and bottom with heavy weight photodegradable polypropylene netting having ultraviolet additives to delay the breakdown. The mat shall be sewn together on 1.5 inch centers with polyester thread.

3. Physical Properties

The Erosion Control and Vegetation Mat shall also conform to the following physical properties.

PROPERTY	MARV (a)
Weight (lbs./sq. yd.)	0.5
Polypropylene Netting Opening Dimensions (in)	5/8 x 5/8

4. Shipping and storage

The ECRM shall be shipped/transported in rolls wrapped with a cover for protection from moisture, dust, dirt, debris, and ultraviolet light. The cover shall be maintained undisturbed to the maximum extend possible before placement.

Each roll of ECRM shall be labeled or tagged to clearly identify the manufacturer, brand, type, and the individual production run.

Material Specification 511—Steel Piles

1. Scope

The specification covers the type and quality of steel piles.

2. Bearing piles

Steel bearing piles shall be structural steel H-piles conforming to the requirements of ASTM A 36.

The required length of pile may be fabricated by butt-welding shorter lengths of pile stock. Unless otherwise specified, the cross-section of each pile shall be constant throughout its length. The axis

of the pile shall be straight, and the number of welded joints in the length of the pile shall be as few as practicable. Pieces below the top piece shall have a minimum length of 10 feet.

3. Sheet piles

Steel sheet piles shall conform to the requirements of ASTM A 328, A 572, or A 690.

Fabrication of piles from shorter lengths of pile stock is not permitted.

Material Specification 512—Wood Piles

1. Scope

This specification covers the quality of wood piles.

2. Quality of piles

The piles shall conform to the requirements of ASTM D 25 for the specified classes and sizes of piles.

3. Treatment

Piles shall be treated with the specified type and amount of preservative and in conformance with the requirements of Material Specification 585.

4. Marking

Each treated pile delivered to the job site shall be marked as specified in Material Specification 585.

Material Specification 547—Plastic Pipe

1. Scope

This specification covers the quality of Poly Vinyl Chloride (PVC), Polyethylene (PE), High Density Polyethylene (HDPE), and Acrylonitrile-Butadiene-Styrene (ABS) plastic pipe, fittings, and joint materials.

2. Material

Pipe—The pipe shall be as uniform as commercially practicable in color, opaqueness, density, and other specified physical properties. It shall be free from visible cracks, holes, foreign inclusions, or other defects. The dimensions of the pipe shall be measured as prescribed in ASTM D 2122.

Unless otherwise specified, the pipe shall conform to the requirements listed in this specification and the applicable reference specifications in table 547–2, the requirements specified in Construction Specification 45, Plastic Pipe, and the requirements shown on the drawings.

Fittings and joints—Fittings and joints shall be of a schedule, SDR or DR, pressure class, external load carrying capacity, or pipe stiffness that equals or exceeds that of the plastic pipe. The dimensions of fittings and joints shall be compatible with the pipe and measured in accordance with ASTM D 2122. Joint and fitting material shall be compatible with the pipe material. The joints and fittings shall be as uniform as commercially practicable in color, opaqueness, density, and other specified physical properties. It shall be free from visible cracks, holes, foreign inclusions, or other defects.

Fittings and joints shall conform to the requirements listed in this specification, the requirements of the applicable specification referenced in the ASTM or AWWA specification for the pipe, the requirements specified in Construction Specification 45, and the requirements shown on the drawings.

Solvents—Solvents for solvent welded pipe joints shall be compatible with the plastic pipe used and shall conform to the requirements of the applicable specification referenced in the ASTM or AWWA specification for the pipe, fitting, or joint.

Gaskets—Rubber gaskets for pipe joints shall conform to the requirements of ASTM F 477, Elastomeric Seals (Gaskets) for Jointing Plastic Pipe.

3. Perforations

When perforated pipe is specified, perforations shall conform to the following requirements unless otherwise specified in Construction Specification 45 or shown on the drawings:

- Perforations shall be either circular or slots.
- Circular perforations shall be $1/4 \pm 1/16$ -inch diameter holes arranged in rows parallel to the axis of the pipe. Perforations shall be evenly spaced along each row such that the center-to-center distance between perforations is not less than eight times the perforation diameter. Perforations may appear at the ends of short and random lengths. The minimum perforation opening per foot of pipe shall be as shown in table 547–1.

Table 547–1 Perforations

Nominal pipe size (inches)	Minimum number of rows		Minimum opening/foot (square inches)
	circular	slot	
4	2	2	0.22
6	4	2	0.44
8	4	2	0.44
10	4	2	0.44
12	6	2	0.66

Rows shall be arranged in two equal groups at equal distance from the bottom on each side of the vertical centerline of the pipe. The lowermost rows of perforations shall be separated by an arc of not less than 60 degrees or more than 125 degrees. The uppermost rows of perforations shall be separated by an arc not to exceed 166 degrees. The spacing of rows between these limits shall be uniform. The minimum number of rows shall be as shown in table 547–1.

- c. Slot perforations shall be symmetrically located in two rows, one on each side of the pipe centerline. Slot perforations shall be located within the lower quadrants of the pipe with slots no wider than 1/8 inch and spaced not to exceed 11 times the perforation width.

Minimum perforation opening per lineal foot of pipe shall be as shown in table 547-1.

- d. On both the inside and outside of the pipe, perforations shall be free of cuttings or frayed edges and of any material that would reduce the effective opening.

Table 547-2 Pipe specification

Pipe	Specification
Poly vinyl chloride (PVC) pipe	
Plastic pipe - Schedules 40, 80, 120	ASTM D 1785
	ASTM D 2466
Pressure rated pipe - SDR Series.....	AWWA C 900
	ASTM D 2241
Plastic drain, waste, and vent pipe and fittings	ASTM D 2665
Joints for IPS PVC pipe using solvent weld cement	ASTM D 2672
Composite sewer pipe	ASTM D 2680
Type PSM PVC sewer pipe and fittings.....	ASTM D 3034
Large-diameter gravity sewer pipe and fittings.....	ASTM F 679
Smooth-Wall Underdrain Systems for Highway, Airport, and Similar Drainage.....	ASTM F 758
Type PS-46 gravity flow sewer pipe and fittings	ASTM F 789
Profile gravity sewer pipe and fittings based on controlled inside diameter.....	ASTM F 794
Corrugated sewer pipe with a smooth interior and fittings	ASTM F 949
Pressure pipe, 4-inch through 12-inch for water distribution.....	AWWA C 900
Water transmission pipe, nominal diameters 14-inch through 36-inch.....	AWWA C 905
Polyethylene (PE) plastic pipe	
Schedule 40	ASTM D 2104
SDR-PR based on controlled inside diameter.....	ASTM D 2239
Schedules 40 and 80 Based on outside diameter.....	ASTM D 2447
SDR-PR based on controlled outside diameter	ASTM D 3035
High density polyethylene (HDPE) plastic pipe	
Plastic pipe and fittings	ASTM D 3350
SDR-PR based on controlled outside diameter	ASTM F 714
Plastic moldings and extrusion compounds	ASTM D 1248
Heat joining polyolefin pipe and fittings.....	ASTM D 2657
Acrylonitrile-butadiene-styrene (ABS) pipe	
Plastic pipe, schedules 40 and 80	ASTM D 1527
Plastic pipe, SDR-PR	ASTM D 2282
Schedule 40 plastic drain, waste, and vent pipe	ASTM D 2661
Composite sewer pipe	ASTM D 2680
Sewer pipe and fittings.....	ASTM D 2751

Material Specification 551—Coated Corrugated Steel Pipe

1. Scope

This specification covers the quality of zinc-coated, aluminum-coated, aluminum-zinc alloy-coated, and polymer-coated corrugated steel pipe and fittings.

2. Pipe

All pipe shall be metallic zinc-coated, aluminum-coated, or aluminum-zinc alloy-coated corrugated steel pipe and fittings conforming to the requirements of ASTM A 742, A 760, A 761, A 762, A 849, A 875, A 885, and A 929 for the specified type, class, fabrication of pipe and coating, and to the following additional requirements:

- a. When closed riveted pipe is specified:
 - (1) Pipe shall be fabricated with circumferential seam rivet spacing that does not exceed 3 inches except that 12 rivets are sufficient to secure the circumferential seams in 12-inch pipe.
 - (2) Longitudinal seams that will be within the coverage area of a coupling band, the rivets shall have flat heads or the rivets and holes shall be omitted and the seams shall be connected by welding to provide a minimum of obstruction to the seating of the coupling bands.
- b. Double riveting or double spot welding for pipe less than 42 inches in diameter may be required. When double riveting or double spot welding is specified, the riveting or welding shall be performed in a manner specified for pipe 42 inches or greater in diameter.

3. Coatings

Coatings described herein, unless otherwise specified, equally refer to the inside and outside pipe surfaces.

When coatings in addition to metallic coatings are specified, they shall conform to the requirements of ASTM A 742, A 760, A 761, A 762, A 849, A 875, A 885, and A 929 for the specified type.

Polymer-coated pipe, unless otherwise specified on the drawings or in the construction specifications, shall be coated on each side with a minimum thickness of 0.01 inches (10 mils), designated as grade 10/10 in ASTM A 762.

4. Coupling bands

Coupling bands are to be provided for each section of pipe. The hardware for fastening the coupling band tightly to the connecting pipe shall be fabricated to permit tightening sufficiently to provide the required joint tensile strength and, if required, watertightness without failure of its fastening.

Gaskets, if specified, are to be provided for each coupling band. The fabrication of coupling bands and fastening hardware, in addition to the above, shall be sufficient to provide the required gasket seating without warping, twisting, or bending.

5. Fittings

Fittings shall be fabricated from steel conforming to ASTM A 742, A 849, A 875, A 885, and A 929. The coating of fittings shall be the same as that specified for the contiguous corrugated coated pipe.

Welded surfaces and adjacent surfaces damaged during welding shall be treated by removing all flux residue and weld splatter. The affected surfaces shall be cleaned to bright metal by sand blasting, power disk sanding, or wire brushing. The cleaned area shall extend at least 0.5 inch into the undamaged section of the coated area. Repair and coating application of damaged and uncoated pipe surface areas shall be in accordance with ASTM A 780.

Material Specification 581—Metal

1. Scope

This specification covers the quality of steel and aluminum alloys.

2. Structural steel

- Structural steel shall conform to the requirements of ASTM A 36.
- High-strength low-alloy structural steel shall conform to ASTM A 242 or A 588.
- Carbon steel plates of structural quality to be bent, formed, or shaped cold shall conform the ASTM A 283, Grade C.
- Carbon steel sheets of structural quality shall conform to ASTM Standard A 1011, Grade 40, or A 1008, Grade 40.
- Carbon steel strip of structural quality shall conform to ASTM Standard A 1011, Grade 36.

3. Commercial or merchant quality steel

Commercial or merchant quality steel shall conform to the requirements of the applicable ASTM listed below:

Product	ASTM standards
Carbon steel bars	A 575, Grade M 1015 to Grade M 1031
Carbon steel sheets	A 1011
Carbon steel strips	A 1011
Zinc-coated carbon steel sheets	A 653 or A 924

4. Aluminum alloy

Aluminum alloy products shall conform to the requirements of the applicable ASTM standard listed below. Unless otherwise specified, alloy 6061-T6 shall be used.

Product	ASTM standard
Standard structural shape	B 308
Extruded structural pipe and tube.....	B 429
Extruded bars, rods, shapes, and tubes	B 221
Drawn seamless tubes	B 210
Rolled or cold-finished bars, rods, and wire	B 211
Sheet and plate	B 209

5. Bolts

Steel bolts shall conform to the requirements of ASTM Standard A 307. If high-strength bolts are specified, they shall conform to the requirements of ASTM A 325.

When galvanized or zinc-coated bolts are specified, the zinc coating shall conform to the requirements of ASTM Standard A 153 except that bolts 0.5 inch or less in diameter may be coated with electro-deposited zinc or cadmium coating conforming to the requirements of ASTM Standard B 633, Service Condition SC 3, or ASTM B 766, unless otherwise specified.

6. Rivets

Unless otherwise specified, steel rivets shall conform to the requirements of ASTM Specification A 31, Grade B. Unless otherwise specified, aluminum alloy rivets shall be Alloy 6061 conforming to the requirements of ASTM Standard B 316.

7. Welding electrodes

Steel welding electrodes shall conform to the requirements of American Welding Society Specification AWS A5.1, "Specification for Mild Steel Covered Arc-Welding Electrodes," except that they shall be uniformly and heavily coated (not washed) and shall be of such a nature that the coating does not chip or peel while being used with the maximum amperage specified by the manufacturer.

Aluminum welding electrodes shall conform to the requirements of American Welding Society Specification AWS A5.10, "Specification for Aluminum and Aluminum-Alloy Welding Rods and Bare Electrodes."

Material Specification 582—Galvanizing

1. Scope

This specification covers the quality of zinc coatings applied to iron and steel productions.

2. Quality

Zinc coatings shall conform to the requirements of ASTM A 123 for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products or as otherwise specified in the items of work and construction details of the Construction Specification.

ASTM A 123 covers both fabricated and nonfabricated products; e.g., assembled steel products, structural steel fabrications, large tubes already bent or welded before galvanizing, and wire work fabricated from noncoated steel wire. It also covers steel forgings and iron castings incorporated into pieces fabricated before galvanizing or which are too large to be centrifuged (or otherwise handled to remove excess galvanizing bath metal).

Items to be centrifuged or otherwise handled to remove excess zinc shall meet the requirements of ASTM A 153, except bolts, screws, and other fasteners 0.5 inch or less in diameter may be coated with electro-deposited zinc or cadmium coating conforming to the requirements of ASTM B 766, coating thickness Class 5, Type III, or ASTM B 633, Service Condition SC-3, unless otherwise specified.

Material Specification 584—Structural Timber and Lumber

1. Scope

The specification covers the quality of structural timber, lumber, and plywood.

2. Grading

Structural timber and lumber shall be graded in accordance with the grading rules and standards, applicable to the specified species adopted by a lumber grading or inspection bureau or agency recognized as being competent and that conform to the basic principles of ASTM Standard D 245. The material supplied according to the commercial grading rules shall be of equal or greater stress value than the specified stress-grade.

3. Quality of material

All material shall be sound wood free from decay and disease damage. Boxed heart pieces of Douglas fir or redwood shall not be used in stringers, floor beams, caps, posts, sills, or other principal structural members. Boxed heart pieces are defined as timber so sawed that at any section in the length of a sawed piece the pith lies entirely inside the four faces.

4. Heartwood requirements

All timber and lumber specified for use without preservative treatment shall contain a minimum of 75 percent heartwood on any diameter or on any side or edge, measured at the point where the greatest amount of sapwood occurs. This requirement shall not apply to timber and lumber for which pressure treatment with wood preservative is specified.

5. Sizes

The sizes specified are nominal sizes. Unless otherwise specified, the material shall be furnished in American Standard dressed sizes.

6. Marking

Each piece of timber and lumber shall be legibly stamped or branded with an official grade identification. Plywood shall be legibly stamped with an official mark designating the grade, type, and surface finish as described in the cited Product Standard.

Material Specification 585—Wood Preservatives and Treatment

1. Scope

This specification covers the quality of wood preservatives and methods of treatment of wood products.

2. Treating practices

Treating practices and sampling, inspection, and test procedures shall conform to the requirements of ASTM D 1760, "Standard Specification for Pressure Treatment of Timber Products."

3. Preservatives

The wood shall be treated with the specified type of preservative. Wood preservatives shall conform to the requirements of the applicable specifications list in ASTM D 1760.

4. Quality of treated material

Treated lumber, timber, piles, poles, or posts shall be free from heat checks, water bursts, excessive checking, results of chafing, or from any other damage or defects that would impair their usefulness or durability for the purpose intended. The use of s-irons is not permitted. Holes bored for tests shall be filled with tight fitting, treated wood plugs.

5. Marking

Each treated wood item delivered to the job site shall be marked as specified in ASTM D 1760, unless otherwise specified.

Material Specification 592—Geotextile

1. Scope

This specification covers the quality of geotextiles.

2. General requirements

Fibers (threads and yarns) used in the manufacture of geotextile shall consist of synthetic polymers composed of a minimum of 85 percent by weight polypropylenes, polyesters, polyamides, polyethylene, polyolefins, or polyvinylidene-chlorides. They shall be formed into a stable network of filaments or yarns retaining dimensional stability relative to each other. The geo-textile shall be free of defects and conform to the physical requirements in tables 592–1 and 592–2. The geotextile shall be free of any chemical treatment or coating that significantly reduces its porosity. Fibers shall contain stabilizers and/or inhibitors to enhance resistance to ultraviolet light.

Thread used for factory or field sewing shall be of contrasting color to the fabric and made of high strength polypropylene, polyester, or polyamide thread. Thread shall be as resistant to ultraviolet light as the geotextile being sewn.

3. Classification

Geotextiles shall be classified based on the method used to place the threads or yarns forming the fabric. The geotextiles will be grouped into woven and nonwoven types.

Woven—Fabrics formed by the uniform and regular interweaving of the threads or yarns in two directions. Woven fabrics shall be manufactured from monofilament yarn formed into a uniform pattern with distinct and measurable openings, retaining their position relative to each other. The edges of fabric shall be selvaged or otherwise finished to prevent the outer yarn from unraveling.

Nonwoven—Fabrics formed by a random placement of threads in a mat and bonded by heat-bonding, resin-bonding, or needle punching. Nonwoven fabrics shall be manufactured from individual fibers formed into a random pattern with distinct, but variable small openings, retaining their position

relative to each other when bonded by needle punching, heat, or resin bonding. The use of nonwovens other than the needle punched geotextiles is somewhat restricted (see note 3 of table 592–2).

4. Sampling and testing

The geotextile shall meet the specified requirements (table 592–1 or 592–2) for the product style shown on the label. Product properties as listed in the latest edition of the "Specifiers Guide," Geotechnical Fabrics Report, (Industrial Fabrics Association International, 1801 County Road BW, Roseville, MN 55113-4061) and that represent minimum average roll values, are acceptable documentation that the product style meets the requirements of these specifications.

For products that do not appear in the above directory or do not have minimum average roll values listed, typical test data from the identified production run of the geotextile will be required for each of the specified tests (tables 592–1 or 592–2) as covered under clause AGAR 452.236-76.

5. Shipping and storage

The geotextile shall be shipped/transported in rolls wrapped with a cover for protection from moisture, dust, dirt, debris, and ultraviolet light. The cover shall be maintained undisturbed to the maximum extend possible before placement.

Each roll of geotextile shall be labeled or tagged to clearly identify the brand, class, and the individual production run in accordance with ASTM D 4873.

Table 592–1 Requirements for woven geotextiles

Property	Test method	Class I	Class II & III	Class IV
Tensile strength (pounds) ^{1/}	ASTM D 4632 grab test	200 minimum in any principal direction	120 minimum in any principal direction	180 minimum in any principal direction
Elongation at failure (percent) ^{1/}	ASTM D 4632 grab test	<50	<50	<50
Puncture (pounds) ^{1/}	ASTM D 4833	90 minimum	60 minimum	60 minimum
Ultraviolet light (% residual tensile strength)	ASTM D 4355 150-hr exposure	70 minimum	70 minimum	70 minimum
Apparent opening size (AOS)	ASTM D 4751	As specified, but no smaller than 0.212 mm (#70) ^{2/}	As specified, but no smaller than 0.212 mm (#70) ^{2/}	As specified, but no smaller than 0.212 mm (#70) ^{2/}
Percent open area (percent)	CWO-02215-86	4.0 minimum	4.0 minimum	1.0 minimum
Permittivity sec ⁻¹	ASTM D 4491	0.10 minimum	0.10 minimum	0.10 minimum

1/ Minimum average roll value (weakest principal direction).

2/ U.S. standard sieve size.

Note: CWO is a USACE reference.

Table 592–2 Requirements for nonwoven geotextiles

Property	Test method	Class I	Class II	Class III	Class IV ^{3/}
Tensile strength (lb) ^{1/}	ASTMD 4632 grab test	180 minimum	120 minimum	90 minimum	115 minimum
Elongation at failure (%) ^{1/}	ASTMD 4632	≥ 50	≥ 50	≥ 50	≥ 50
Puncture (pounds)	ASTMD 4833	80 minimum	60 minimum	40 minimum	40 minimum
Ultraviolet light (% residual tensile strength)	ASTMD 4355 150-hr exposure	70 minimum	70 minimum	70 minimum	70 minimum
Apparent opening size (AOS)	ASTMD 4751	As specified max. #40 ^{2/}	As specified max. #40 ^{2/}	As specified max. #40 ^{2/}	As specified max. #40 ^{2/}
Permittivity sec ⁻¹	ASTMD 4491	0.70 minimum	0.70 minimum	0.70 minimum	0.10 minimum

1/ Minimum average roll value (weakest principal direction).

2/ U.S. standard sieve size.

3/ Heat-bonded or resin-bonded geotextile may be used for classes III and IV. They are particularly well suited to class IV. Needle-punched geotextiles are required for all other classes.